

Parametrix No. 553-7443-006

Brent Copes REHS/RS Central District Health Department Environmental Health Specialist Senior Community & Environmental Health

Re: Bennett Road Landfill Groundwater Monitoring Plan

Dear Mr. Copes:

The Groundwater Monitoring Plan has been prepared by Parametrix on behalf of Elmore County for the Bennett Road Landfill. The Groundwater Monitoring Plan is comprised of the 2024 Bennett Road Landfill Hydrogeologic Characterization Work Plan (plan) with an attached Sampling and Analysis Plan. This plan meets federal and state requirements for a Groundwater Monitoring Plan and is provided in accordance with Idaho Administrative Code (IDAPA) 58.01.06, Idaho Statutes §39-7409 through §39-74), and Federal Regulation 40, Code of Federal Regulations (CFR) 258 under the regulatory supervision of the Idaho Department of Environmental Quality (IDEQ).

Sincerely,

#### Parametrix

Tiffany Neier, Project Manager

Shira DeGrood, Lead Geologist

cc: Project File

Matthew Beeter, Solid Waste Program Manager, Idaho Department of Environmental Quality

Deb Ireland, Landfill Supervisor, Elmore County

Al Hofer, Elmore County Commissioner



# 2024 Bennett Road Landfill Hydrogeologic Characterization Work Plan – DRAFT

Prepared for Elmore County



July 2024



# 2024 Bennett Road Landfill Hydrogeologic Characterization Work Plan

Prepared for

**Elmore County** 150 S 4th E Street Mountain Home, ID 83647

Prepared by

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# Citation

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# Certification

The technical material and data contained in this document were prepared under the supervision and direction of the undersigned, whose seal, as a professional engineer licensed to practice as such, is affixed below.

Prepared by Shira DeGrood, PG	
Checked by Michael Brady, LG, LHG	
Approved by Tiffany Neier, PE	

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# **Acronyms and Abbreviations**

BRL	Bennett Road Landfill
C&D	construction and demolition
CFR	Code of Federal Regulations
County	Elmore County
ft amsl	feet above mean sea level
ft bgs	feet below ground surface
IDAPA	Idaho Administrative Code
IDEQ	Idaho Department of Environmental Quality
IDWR	Idaho Department of Water Resources
MSW	municipal solid waste
NAD83	North American Datum of 1983
NAVD88	North American Vertical Datum of 1988
POC	point of compliance
QA/QC	quality assurance and quality control
USGS	United States Geological Survey

## **1.** General Information

Elmore County (County) owns and operates the Bennett Road Landfill (BRL) in accordance with Idaho Administrative Code (IDAPA) 58.01.06, Idaho Solid Waste Facilities Act (§39-7410), Federal Regulation 40, Code of Federal Regulations (CFR) 258.51, and 40 CFR 257.24 under the regulatory supervision of the Idaho Department of Environmental Quality (IDEQ).

The BRL is located at 6100 SE County Landfill Road in Mountain Home, Idaho (Figure 1), and encompasses a transfer station, unlined municipal solid waste (MSW) landfill (Cell 1), and a construction and demolition (C&D) landfill. Figure 2 displays the details of the facility.

The County currently accepts less than approximately 20 tons of MSW per day into Cell 1 and is currently exempt from the requirements of subparts D and E of 40 CFR 258. The County is planning for the facility to accept a greater amount of in-County waste and a smaller amount of out-of-County waste that would put the waste acceptance over the small community exemption of 20 tons per day/7,300 tons per year. The County is currently designing a new expansion, Cell 2, which will be lined. To support the greater amount of waste acceptance, Parametrix was retained to develop a hydrogeologic characterization work plan to satisfy the Groundwater Monitoring Application of IDAPA 58 and requirements of subpart E of 40 CFR 258.

# 2. Physical Setting

The BRL is located in central Elmore County located approximately 5 miles southeast of Mountain Home, Idaho. The BRL is located within the western Snake River basin. The basin is a broad downwarped structural basin developed by migration of the Yellowstone hot spot. The basin is bounded by low mountain ranges such as the Mount Bennett Hills, located approximately 14 miles northeast of the BRL. The Snake River is located approximately 10 miles to the south and flows northwest towards the Columbia River. Several smaller creeks, such as Bennett Creek, flow from Bennett Mountain south towards the Snake River. The West Tributary of Dry Creek has several ephemeral branches that cross the BRL facility boundary flowing from the north southwesterly towards the Snake River.

The elevation of the BRL is approximately 3,200 feet above mean sea level (ft amsl) and the area surrounding the landfill is entirely flat. The land slopes gently from north to south towards the Snake River which is at an elevation of approximately 2,500 ft amsl south of the BRL.

## 2.1 Soils

Soils in the area of the BRL are mapped as Colthorp-Kunaton complex, 0 to 8% slopes (NRCS 2024) which are described as stony silt loams and silty clay loams overlying duripan. The parent material is mixed alluvium or loess over basalt bedrock. The loam soils are generally well drained, less than 18 inches thick, and overly a 5- to 16-inch thick duripan.

Test pits excavated adjacent to the proposed landfill footprint indicated bedrock or duripan near the surface at the southeast corner of Cell 1 (Parametrix 2016). Approximate test pit locations are provided in Figure 2. Soils encountered in three test pits east of Cell 2 ranged from clay to clayey silt. The sandy silts were observed to consistently have a slightly higher proportion of silt in their lower portions. The uppermost Unit 1 had approximately 50% silt and 50% sand, and the underlying Unit 2 had approximately 60% silt and 40% sand. Permeability of the surficial soils based on four undisturbed samples ranged from 4.1 to  $7.4 \times 10^{-04}$  centimeters per second.

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## 2.2 Geology

#### 2.2.1 Regional Geology

A series of northwest trending normal faults are mapped throughout the western Snake River basin (Malde et al 1963, Lewis et al 2012). None of the faults mapped in the vicinity of the BRL are Quaternary in age (USGS 2014, Algermissen 1990).

The Quaternary Bruneau Formation of the Idaho Group (Parliman 1983, Lewis et al 2012, Malde et al 1963) is mapped at land surface at the BRL. The Bruneau Formation has a total thickness of up to 800 feet and consists primarily of basaltic lava flows and sedimentary deposits including some fan deposits and consolidated detrital material. The volcanic rocks of the Bruneau Formation were extruded from vents between the Snake River and the Mount Bennett Hills, filling canyons eroded into the Snake River basin area. The basalts are a series of discontinuous flows that are more permeable at their top and bottom than in the centers, and are described as vesicular olivine basalt, dark gray to black, weathering to reddish gray-brown. Well-developed columnar jointing is present throughout, and the fractured and vesicular nature of the Bruneau Formation basalts cause them to have high permeabilities (Ralston and Chapman 1968).

Within the Bruneau Formation, fan deposits consist primarily of coarse sands derived from decayed granitic rocks and consolidated detrital material consists primarily of low permeability massive white-weathering lakebed deposits of fine silt, clay, diatomite, and minor amounts of sand.

The Glenns Ferry Formation lies underneath the Bruneau Formation and is characterized by thick intertonguing deposits of lake and stream sediments including darker colored clay/shale deposits.

#### 2.2.2 Local Geology

#### **2.2.2.1** Data Sources

Available information on local geology and depth to groundwater includes facility-specific sources of information and well logs from the Idaho Department of Water Resources (IDWR 2024b). Available bore logs, well logs, and test pit descriptions are provided in Appendix A and locations are depicted in Figure 2.

Local geology data sources:

- Five borings (BH-1 through BH-5) advanced to depths of 18.5 to 32.5 feet below ground surface (ft bgs) as part of the 2016 Arid Design Demonstration (Parametrix 2016).
- Boring BH-6, advanced to a depth of 60 ft bgs and was completed as a monitoring well as part of the 2016 Arid Design Demonstration (Parametrix 2016).
- Ten test pits (TP88-1 through TP88-10) excavated to depths of up to 20 ft bgs in 1998. Located within, north, and east of the BRL.
- Three test pits (TP-1 through TP-3) excavated to depths of 6 to 8 ft bgs located east of the Cell 2 location as part of the 2016 Arid Design Demonstration (Parametrix 2016).
- The Facility Supply Well 04S-07E-14AAA1 owned by Elmore County. The United States Geological Survey (USGS) currently measures groundwater elevations at this well semiannually as part of the Mountain Home Plateau Hydrologic Investigation (IDWR 2024a; USGS 2024a).
- The House wells (04S-07E-13AAD1 through 04S-07E-13AAD4) are four historical domestic boreholes/wells drilled between 1965 and 1969 on the parcel currently owned by Elmore County. The well logs include subsurface information up to 2,045 ft bgs near the facility.

#### 2.2.2.2 Summary of Geologic Observations

Depth to bedrock ranged from 8 ft bgs near the southern boundary of proposed Cell 2 (BH-5) and 22.5 (BH-2). The soils consisted of sandy silts (ML) with some intermittent zones of lightly to moderately cemented material. The shallow soils generally consist of nonplastic sandy silt, ranging in thickness from 8.5 to 22.5 feet, overlying basalt rock, although slightly plastic clay was observed in one test pit (TP-1) at a depth of 5 to 6 ft bgs.

The Facility Supply Well was drilled to a depth of 610 ft bgs and encountered clay down to 20 ft bgs, then gray basalt to 67 ft bgs . The remainder of this boring lost all drill cutting returns/circulation using the air rotary and was interpreted to be basalt by the driller.

The well logs for the House wells show the subsurface geology is comprised of basalt of the Bruneau Formation down to approximately 504 to 590 ft bgs prior to encountering the Glenns Ferry Formation. The upper portion of the Glenns Ferry Formation consists of blue shale/clay and black-gray sand down with interflows of basalt to at least 2,045 ft bgs.

#### 2.3 Hydrogeology

#### 2.3.1 Regional Hydrogeology

A perched groundwater system is present in the Mountain Home area west of the BRL, with depths to groundwater of 10 to 200 ft bgs (IDWR 1994). The perched aquifer occurs mostly in Quaternary Alluvium sands, silts and clays, but also may be present within the Bruneau Formation basalts. The eastern limit of the perched aquifer is mapped approximately 2 miles west of the BRL (Parliman 1983, Norton et al 1982).

The regional aquifer is present within the Bruneau Formation basalts at approximate elevations of approximately 2,800 ft amsl in the vicinity of the BRL (IDRW 1994, Ralston and Chapman 1968). The Bruneau Formation is described as being the primary aquifer in the area with groundwater encountered in the interflow zone and thin sand beds immediately below the basalt. The aquifer has yields ranging from 10 to 3,500 gallons per minute. Regional groundwater flow in the vicinity of the BRL generally follows the slope of the topography and is toward the south-southwest from the Mount Bennett Hills area toward the Snake River (Ralston and Chapman 1968, Parliman 1983).

Geothermal water is present below the regional aquifer. Often the geothermal water has a higher hydraulic head and therefore discharges deep groundwater to the regional aquifer (Lindholm 1996).

The BRL is located adjacent to and within the Mountain Home Groundwater Management Area established in 1982 by the Idaho Department of Water Resources (Norton et al 1982, Castelin 1988). This area has been subject to declining water levels due to withdrawals for irrigation and domestic use (IDWR 1994, IDWR 2004).

#### 2.3.1.1 Water Level Declines

The USGS has been tracking water level declines in the Mountain Home Groundwater Management Area and has been measuring water levels at several wells. water levels declined 33 feet between 1989 and 2010 at Well 04S-07E-17CAB1, and water levels declined 47 feet at Well 04S-07E-18AAA1 between 1989 and 2023. Both wells are located approximately 3 miles west of the BRL (USGS 2024b). Assuming that water level declines remain similar, the projected rate of water level decline is approximately 1.5 feet per year. The regional aquifer is confined and water level declines are extreme due to the low recharge rate of the aquifer, excess water uses, and declining pressure heads. 2024 Bennett Road Landfill Hydrogeologic Characterization Work Plan Elmore County

#### 2.3.2 Local Hydrogeology

Groundwater was not encountered in Well BH-6, which is screened in the top of the Bruneau Formation basalt from 40 to 60 ft bgs. The USGS is currently measuring groundwater levels in the Facility Supply Well as part of the regional water level decline evaluation. The depth to groundwater was measured in March 2023 and November 2023 at 432.52 ft bgs and 432.37 ft bgs (2,737.55 and 2,767.70 ft amsl), respectively (USGS 2024a). There is no record of the static water level in the well log at the time of drilling in 1990. Based on the regional study, static water levels in the Facility Supply Well are anticipated to have been 40 to 50 feet higher at the time of drilling.

Warm water and clay were encountered at depths greater than 1,800 ft bgs in the House wells, drilled on the eastern portion of the County-owned parcel. Temperatures were reported at approximately 110 to 120° degrees Fahrenheit in clays (present beginning at 1,805 ft bgs in Well 04S-07E-13AAD3) and hot mud was encountered near the bottom of Well 04S-07E-13AAD2.

#### 2.3.2.1 Specific Capacity Data

Few well logs have well testing information. Well 04S-07E-16BBB1, completed within the Bruneau Formation, was tested at 3,200 gallons per minute for 6 hours and had a total drawdown of 22 feet. This is equivalent to a specific capacity of 145 gallons per minute per foot of drawdown. The USGS identified the specific capacity for wells within Township 4 South, Range 7 East to range from 7 to 360 gallons per minute per foot of drawdown (Young 1977).

## **3.** Current Hydrogeologic Understanding

Parametrix developed two hydrogeologic cross sections (A-A' and B-B') in the vicinity of the BRL based on available well logs and confirmed locations. Well logs used in this assessment are provided in Appendix A. The location of wells and cross-section lines are displayed on Figure 3 and the cross sections are shown in Figures 4 (A-A') and 5 (B-B').

#### 3.1 Data Sources

A well log database search was conducted using the Idaho Department of Water Resources (IDWR) website (IDWR 2024b) and the USGS National Water Information System Mapper (USGS 2024b) to identify wells in the vicinity of the BRL. Wells included in the cross section were limited to locations where a well/borehole log and the well location coordinates could be confirmed.

Wells were identified using the USGS standard methodology for Idaho using township, range, section, and quarter-quarter and followed standard methods for Idaho, as described in USGS Water Resources Investigations Report 83-4062 (Parliman 1983).

There are a limited number of wells in the immediate vicinity of the BRL, see Table 1. Most wells are located several miles west and northwest of the BRL. Therefore, the cross sections extend several miles from the BRL.

Well Identifier	Approximate Distance from BRL (miles)	Owner	Year of Installation	Depth to Water at Time of Drilling (ft bgs)	Drilled Depth (ft bgs)
04S-07E-14AAA1	At Facility	Elmore County	1989	None	610
BH-6	At Facility	Elmore County	2016	None	60
04S-07E-13AAD1	1.0	House	1967	423	635
04S-07E-13AAD2	1.0	House	1967	437	1,840
04S-07E-13AAD3	1.0	House	1969	427	2,045
04S-07E-13AAD4	1.0	House	1965	None	81
04S-07E-33AAA1	3.3	Bergh	1997	440	540
05S-07E-03ADB1	3.9	Fleming	1967	441	592
04S-07E-18AAA1	4.1	Grofsema	1966	305	685
04S-07E-17CAB1	3.8	K.Kon Construction	2003	338	491
04S-07E-16BBB1	3.1	BBKY Corporation	1968	314	569
04S-07E-09DCC1	2.6	Groefsema	1962	350	862
04S-08E-14AAA1	5.7	Ballard	1964	416	583
04S-07E-28BBA1	3.4	Olson	2014	416	735
04S-07E-17CCD1	4.0	Data not available			

Table 1. Summary of Assessed Wells

BRL = Bennet Road Landfill; ft bgs = feet below ground surface

#### **3.2** Hydrostratigraphy

Cross sections are presented in Figures 4 and 5. Figure 3 displays the location of the cross sections and location of wells utilized. Cross section A-A' incorporates the area from the BRL to the southwest, along the regional groundwater gradient. Cross section B-B' incorporates the areas to the west and east of the BRL, generally cross-gradient to the regional groundwater gradient.

The cross sections show the subsurface geology below the BRL is comprised of shallow silt overburden (which may include Bruneau Formation sedimentary deposits) overlying black basalt. The silts are up to approximately 20 feet thick below the BRL. The basalt is the volcanic members of the Bruneau Formation. Yellow clays and sands may be present as thin lenses within the basalt layers. Sedimentary formations including sands, clays, sandstone, and shale of the Glenns Ferry Formation are present underlying the Bruneau Formation.

In the vicinity of the BRL and to the west, the bottom of the Bruneau Formation basalts was observed at depths between approximately 500 and 600 ft bgs. The House wells to the immediate east of the BRL encountered the Glenns Ferry Formation at depths ranging from 504 to 590 ft bgs. The Facility Supply Well did not report encountering the Glenns Ferry Formation down to 600 ft bgs; however, this may be due to loss of returns.

The regional aquifer below the facility occurs within the bottom of the Bruneau Formation and just above fine-grained materials of the Glenns Ferry Formation.

In House Well 04-07E-13AAD2, which was advanced to 1,840 ft bgs, hot mud was reported at the bottom of the borehole, suggesting the presence of geothermal water at deeper depths within the Glenn Springs Formation. Clay with an approximate temperature of 110 to 120° Fahrenheit was present in Well 04-07E-13AAD3 at depths greater than 1,805 ft bgs. As discussed above, geothermal groundwater typically has a higher head than the regional aquifers and discharges upward.

## 3.3 Gradient

The USGS measured groundwater levels in the Facility Supply Well (04S-07E-14AAA1) and in three additional wells located to the west and southwest of the facility in 2023, including Wells 04S-07E-18AAA1, 04S-07E-17CCD1, and 4S-07E-28BBA1 (USGS 2024b). Measured groundwater elevations for these four wells in November 2023 are provided in Figure 3. Local groundwater flow in November 2023 was to the south-southwest at a gradient of 0.0059 feet per foot (31.3 feet per mile) between wells 04S-07E-14AAA1 and 04S-07E-28BBA1. This flow direction and gradient is consistent with previously published groundwater gradients published in 1968 and 1983, provided in Appendix B (Ralston and Chapman 1968, Parliman 1983).

## 4. Proposed Monitoring Well Network

As discussed above, the first groundwater below the BRL is generally encountered between 500 and 600 ft bgs within the Bruneau Formation. There is potential for perched groundwater shallower than the regional aquifer. However, for the purposes of this work plan the wells to be completed will target the regional aquifer system. The gradient within the regional aquifer is to the south-southwest. New monitoring wells will be positioned downgradient of Cell 1 and the future phase Cell 2.

#### 4.1 Well Locations

Parametrix proposes installation of two point of compliance (POC) wells. Proposed locations are provided in Figure 2. MW-1 is proposed to be approximately 400 feet southwest and directly downgradient of Cell 1. MW-2 is proposed to be approximately 350 feet south of Cell 1 and 425 feet southwest directly downgradient of Cell 2.

The current Facility Supply Well is proposed to be used as an upgradient background sample. Figure 2 displays the location of the Facility Supply Well with respect to Cells 1 and 2 and POC wells MW-1 and MW-2. As noted, the Facility Supply Well is upgradient of the BRL operations and should not be impacted by historical land use at the facility. The sample collection location will be a faucet that is connected to the system; the target sampling faucet will be determined prior to the initial sampling event.

It is unknown if any of the House Wells 04-07E-13AAD1 through -13AAD4 remain present. Of these four wells, 04-07E-13AAD3, completed to 2,045 ft bgs northeast of the BRL in 1969, is most likely to still be present. There are no records that the well was properly decommissioned. If the well remains, it is another potential upgradient well that could potentially be utilized to further evaluate background conditions in the vicinity of the BRL.

## 4.2 Depths

The downgradient POC wells will be installed to anticipated total depths of 600 feet bgs. Figures 4 and 5 show the Regional Aquifer is present at this depth and is likely to have a higher hydraulic head when encountered. The Regional Aquifer appears to occur within the interflow zones of the Bruneau Formation and sand beds immediately below occurring near the top of the Glenns Ferry Formation. The anticipated depth to groundwater in the completed wells will be around 450 ft bgs, or elevation 2,750 ft amsl. The wells will be designed to account for anticipated water level declines during the life of the BRL, an anticipated decrease of 45 feet over 30 years.

## 5. Drilling and Construction

The hydrogeologic investigation includes installation of two monitoring wells. Each well will be constructed by an Idaho State licensed driller using air-rotary, dual-rotary drilling, or equivalent approved techniques for a 4-inch-diameter monitoring well completion. Drilling and construction of monitoring wells will be conducted in accordance with IDAPA 37.03.09.

#### 5.1 Start Card

The licensed well driller will apply for and receive a drilling permit (start card) prior to proceeding with drilling.

#### 5.2 Borehole Advancement

Open borehole drilling will be utilized to reach target drilling depths for the two proposed monitoring wells. A telescoping method using air-rotary techniques is proposed for each well using the following strategy:

- A 12-inch-diameter borehole will be advanced from ground surface to the top of bedrock (approximately 20 ft bgs). A 12-inch temporary casing will be installed from ground surface to top of bedrock.
- 2. A 10-inch diameter borehole will be advanced from top of bedrock to the bottom of the borehole. If warranted by drilling conditions, the drillers may step down to an 8-inch diameter drill at depth.

Potable water may be utilized to assist with drilling through the basalt layers. Drill cuttings are flushed from the borehole using water and air in the annular space of the borehole outside of the drill string.

Drill cuttings will be stockpiled adjacent to the borehole, except for samples collected for logging by the field geologist. Drill cutting samples will be collected and stored in ziplock bags labelled with the collection depth. Drill cuttings will be collected at a frequency of 5-foot intervals within water bearing zones and approximately 10-foot intervals in nonwater bearing zones.

The cuttings, discussions with the driller, water levels, and air and water pressure readings will be utilized to log changes in geologic conditions. The field geologist will observe drilling and prepare a boring log documenting field observations and well construction details. Field observations will include:

- Drill penetration rate and quality
- Lithology, color, fracture density, presence/degree of vesicles, weathering, size, angularity, texture, and mineralization
- Observation of moisture (i.e., damp, moist, wet) as allowed by the drilling conditions and methods

If a damp or moist zone is observed, the field geologist may direct the driller to halt drilling and provide access for a manual water level measurement. Drilling may be paused for up to 30 minutes to observe groundwater recharge. If potential recharge is encountered, additional testing will be completed by either pumping or conducting an air-lift test.

The borehole will be completed at approximately 60 feet below the depth of first water encountered.

## 5.3 Well Construction

The monitoring wells will be constructed in accordance with standards provided in IDAPA 37.03.09, Well Construction Standards Rule. Each well will be completed with a sufficient screen to account for potential water level declines over the lifetime of the landfill. The wells will be constructed with 4-inch diameter schedule 80 PVC screens and risers. The screened interval will be packed with silica sand and a bentonite seal will be placed above the pack up to the land surface. The well will be completed with an aboveground lockable monument with three protective bollards. Due to the declining water levels in the aquifer, longer screens will be constructed to ensure the wells can be utilized for the life of the landfill.

Once the well has been constructed, the licensed driller will submit a well driller's report to IDWR.

#### 5.4 Decontamination

Drilling equipment (rods, bits, drill rig, casing, etc.) will be steam cleaned with a high-pressure washer prior to drilling at each location. All drilling equipment that enters the borehole will be steam cleaned with high-pressure water between wells to prevent cross contamination.

## 5.5 Survey

Each of the monitoring points will be surveyed under the supervision of an Idaho State licensed professional land surveyor. The survey will be conducted in IDWR geospatial data standards North American Vertical Datum of 1988 (NAVD88) and North American Datum of 1983 (NAD83) as follows:

- Measurements will be taken at the north side of the top of PVC well casing (well cap/plug removed), the top of the north side of the steel monument, and ground elevation.
- Vertical accuracy will be 0.01 feet or less.
- Horizontal accuracy will be 0.1 feet or less.
- The survey will include measurements of at least two nearby benchmarks and two existing wells to the project datum to demonstrate precision, accuracy, and consistency with prior surveys.
- The survey will include documentation of survey quality assurance, including repeat measurements (closed loop) to demonstrate internal consistency, and documentation of the name, location, accuracy, and precision of the benchmark(s) used in the survey.

#### 5.6 Well Development and Production Rate Testing

Each newly constructed well will be developed by the driller in accordance with IDAPA 37.03.09 25 to remove suspended fines and to promote hydraulic connection with the aquifer. Well development will be performed using surge and bail/pump techniques. If mud rotary is employed, drilling mud will first be pumped out of the well. Water and sediment from the well development will be discharged to the ground of the BRL.

During well development, the well production rate will be determined by operating the pump at sufficient duration to establish production rates.

## 5.7 Dedicated Pump Installation

Each well will be equipped with a dedicated sampling pump, tubing, and wellhead fittings. A highpressure bladder pump will be installed in each monitoring well (QED Well Wizard or equivalent). Pump selection will be based on well completion and water depth at each well.

#### 5.8 Plans and Specifications

Draft plans and specifications have been developed for the work. These will be finalized upon approval of the hydrogeologic work plan. The plans and specifications detail the licensed well driller requirements for construction of the new monitoring wells in accordance with IDAPA 37.03.09 25, well development and testing.

## 6. Hydraulic Testing

Hydraulic testing will be completed at MW-1 and MW-2. The testing will consist of pumping at a constant rate for approximately 1 to 4 hours followed by shutting off the pump and measurement of recovery. Water levels will be monitored using a combination of pressure transducers and manual measurements.

#### 6.1 Transducers

Pressure transducers will be used to continuously monitor water levels in selected wells during the hydraulic testing. Serial numbers for the pressure transducer, type of transducer, and well identification will be recorded on field forms. Following deployment of the transducer, the depth of deployment will be recorded on the field form. The water level should be allowed to equilibrate for 5 minutes and then a manual measurement of the water level will be completed and documented on the field form.

#### 6.2 Manual Measurements

Depth to water will be manually measured in wells to confirm transducer data (for those wells instrumented with pressure transducers). Depths to water will be measured using a water level probe to the nearest one hundredth of a foot (i.e. 0.01 feet).

#### 6.3 Testing procedures

Pumping tests will be conducted to assess aquifer properties. Flow rates will be determined based upon well performance during development. A gas generator or electrical cords will be used to power the pump. Water-level measurements will be taken at various time intervals during the pumping test and recovery. Typically, this is completed every minute for the first 10 minutes, every 5-minutes up to 30 minutes, and every 10 minutes up to 1 hour. After testing for 1 hour, measurements will be collected every hour until completion of the test. Similarly, measurements will be recorded for recovery following shutting the pump off.

The total volume of water removed from the pumping well will also be regularly recorded along with depth to water. Observation wells completed in the same aquifer will be manually measured hourly during the pump test and recovery.

2024 Bennett Road Landfill Hydrogeologic Characterization Work Plan Elmore County

## 6.4 Analysis

Hydraulic testing data will be analyzed to determine aquifer parameters such as hydraulic conductivity by a licensed hydrogeologist. This data will be used in evaluation of flow rates below the BRL resulting from variations in the groundwater gradient.

## 7. Water Quality Testing

Sampling and quality assurance and quality control (QA/QC) procedures will be conducted consistent with procedures documented in the Groundwater Sampling and Analysis Plan (Appendix C). Groundwater will analyzed for the analytical and field parameters summarized below:

- Laboratory Analytics Metals and volatile organic compounds specified in Appendix I of 40 CFR part 258 and additional cations/anions that may be indicators of leachate.
- Field Parameters Temperature, pH, specific conductivity, dissolved oxygen, oxidationreduction potential, visual color, and turbidity.

## 8. Reporting

A summary well installation report will be completed documenting the results of the investigation. The reports will summarize and interpret the following information:

- Geology and hydrogeology below the BRL.
- Preliminary groundwater gradients below the BRL.
- Updated hydrogeologic cross sections below the BRL.
- Well log, drilling, and construction information.
- Well survey information.
- Well hydraulic testing results, hydraulic conductivity evaluation.

Additional reporting following groundwater monitoring is described in detail in the Groundwater Sampling and Analysis Plan (Appendix B).

## 9. References

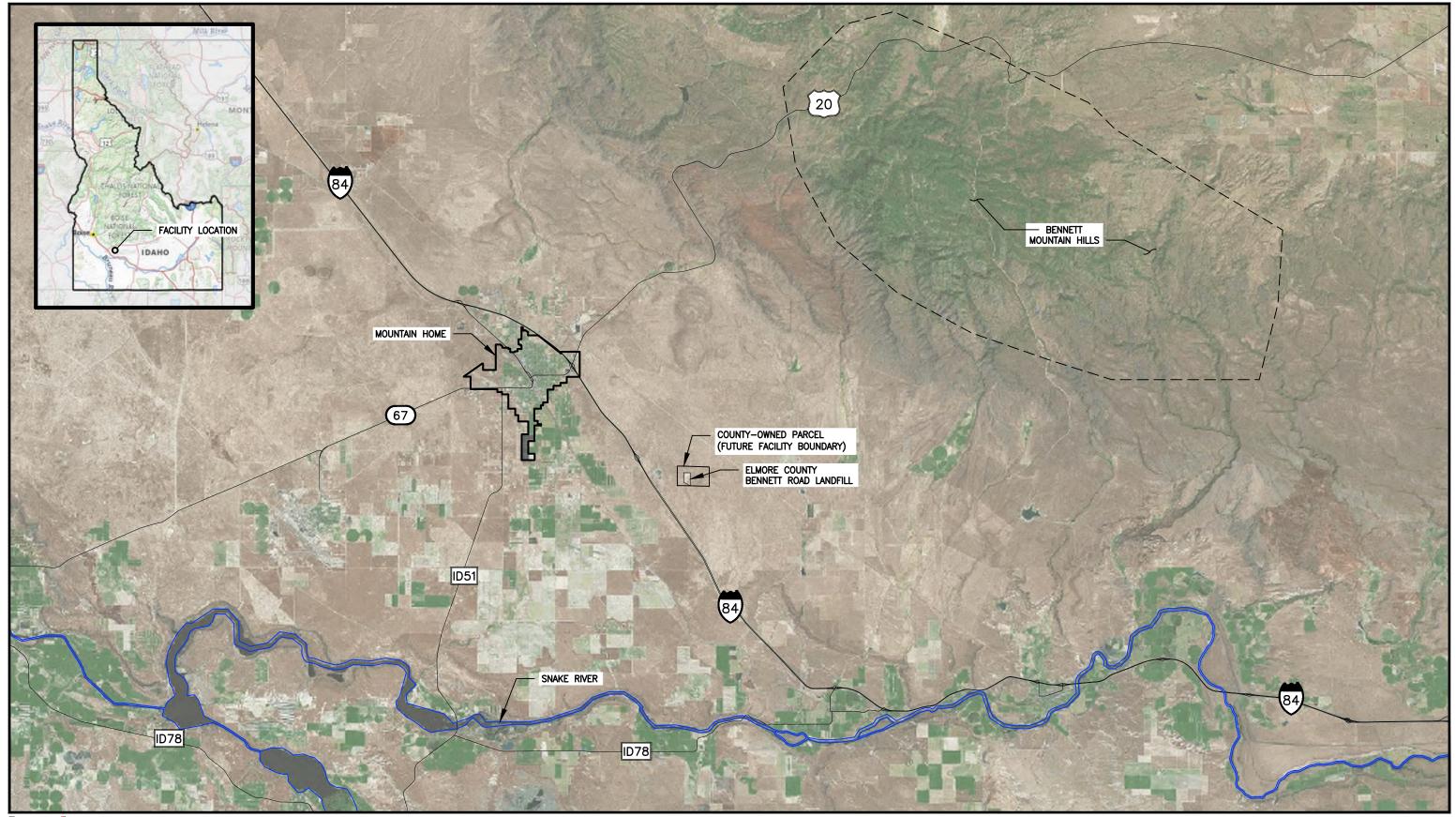
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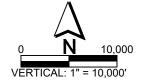
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# **Figures**



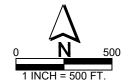
Parametrix DATE: July 12, 2024 FILE: PS07443006T02-FIG\_1



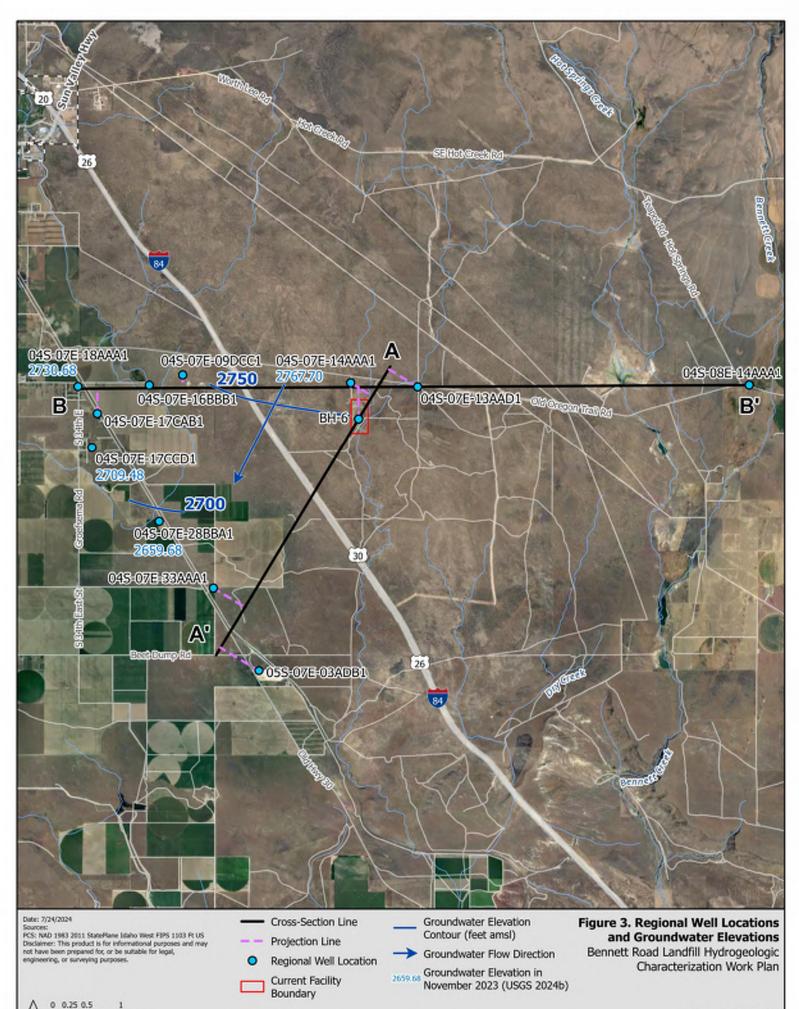
#### **FIGURE 1. SITE VICINITY BENNETT ROAD LANDFILL HYDROGEOLOGIC CHARACTERIZATION WORK PLAN**



DATE: July 12, 2024 FILE: PS07443006T02-FIG\_2

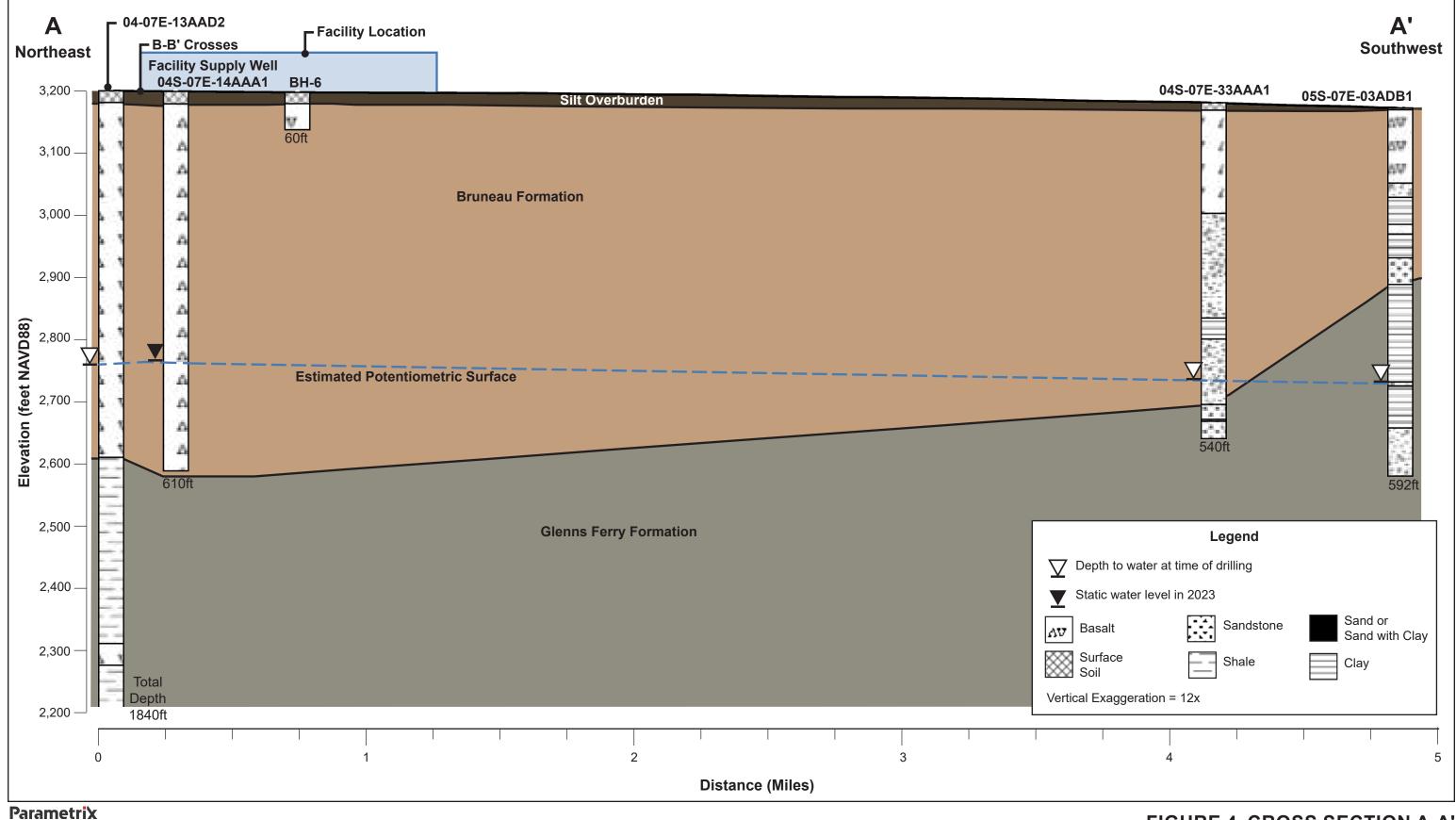


#### **FIGURE 2. FACILITY PLAN BENNETT ROAD LANDFILL HYDROGEOLOGIC CHARACTERIZATION WORK PLAN**



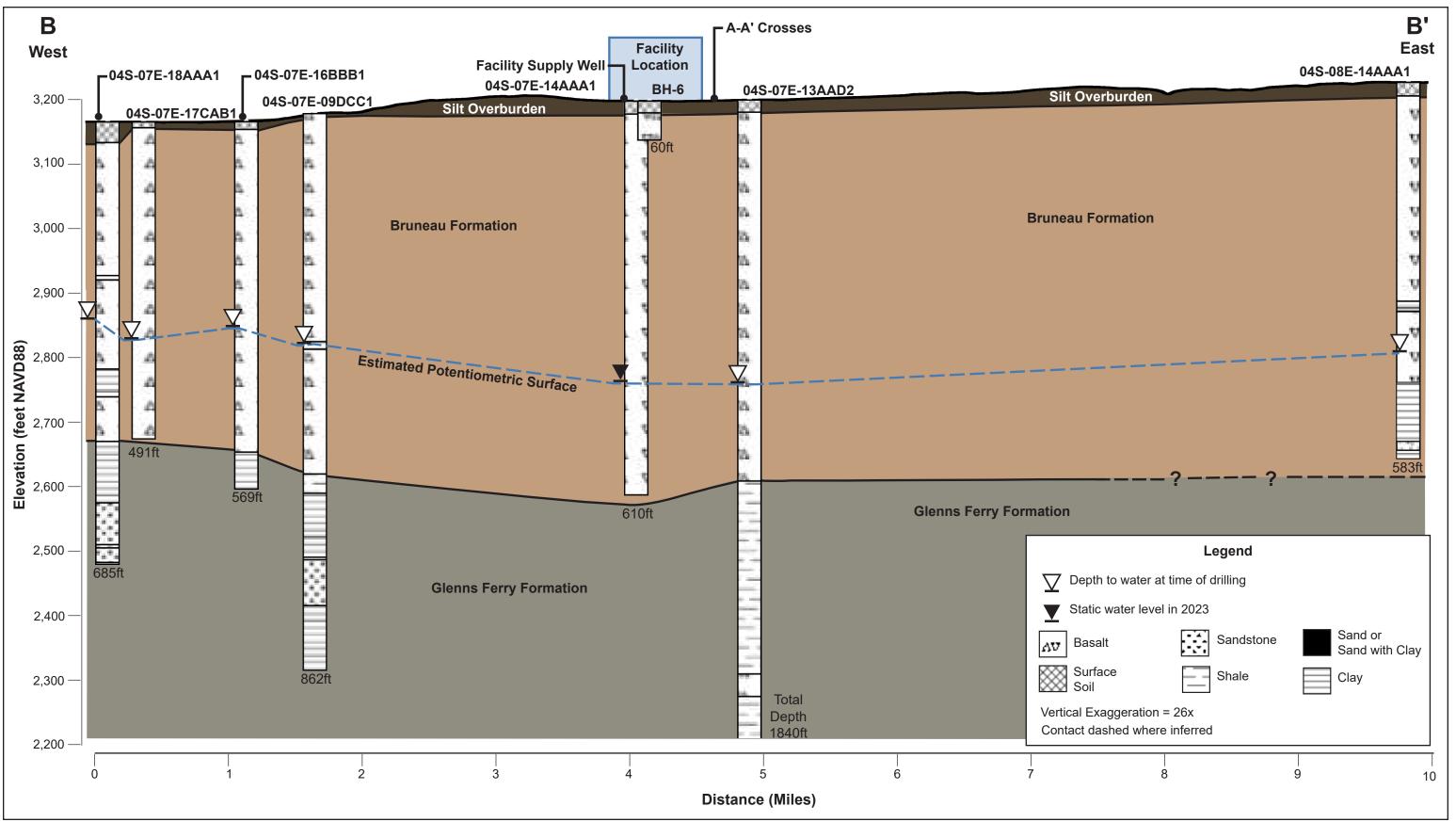
Miles

Elmore County, ID



**BENNETT ROAD LANDFILL HYDROGEOLOGIC CHARACTERIZATION WORK PLAN** 

# FIGURE 4. CROSS SECTION A-A



Parametrix

BENNETT ROAD LANDFILL HYDROGEOLOGIC CHARACTERIZATION WORK PLAN

## FIGURE 5. CROSS SECTION B-B' GIC CHARACTERIZATION WORK PLAN

# **Appendix A**

Monitoring Well, Borehole, and Test Pit Logs

### PROJECT: Mountain Home Landfill LOCATION: Elmore County, Idaho

#### METHOD: Hollow-Stem Auger DATE LOGGED: 11/12/2015 LOGGED BY: Ryan VanLeuven, PE

**BORING NO. B-1** 

#### **GROUNDWATER:**

Groundwater not encountered on 11/12/2015



LATITUDE: 43.076519° LONGITUDE: -115.574414° by Client

ILE NO. 0	2786				PAGE 1 OF 1				BORING NO. B-1
					Bottom of Boring at 31.5 ft on 11/12/2015.				Backfilled hole with bentonite chips.
30	CR-12	100 (88)			as above, except medium to low vesicularity, moderately to widely spaced fractures.				
25	CR-11	88 (37)			Dark gray, moderately hard, medium vesicularity, moderately high to vertical discontinuity angles, moderately rough to rough joint faces, closely to moderately spaced fractures. Driller notes void in rock from 24.5' to 25.0'.				bouncing on rock.
20 <u>SS-9</u>	CR-10	89 (50)			About 40% non-plastic fines; about 40% fine to coarse, subangular to angular sand; about 20% moderately hard, angular basalt gravel to 1". Basalt Rock-	~			slower drilling from 18.0' 20.0'. Switch to HQ core at 20. SS-9; SS sampler
	SS-8	100	18-24-21- 50/0"		Sandy Silt with Gravel (ML)-	-			pushing. Slight auger grinding and
15	ST-7	100							ST-7; crowd pressures from 500 to 1000 psi, difficult ST removal after
	SS-6	100	18-37-42- 50/6"	·   ·   ·   ·   ·   ·   ·   ·   ·   ·	Light particle cementation noted from 12.5' to 14.5'.				pushing.
10	ST-5	100		·   •   .   •	trace of gravel; light brown. Moderate particle cementation noted from 7.5' to 9.5'.			16	ST-5; crowd pressures from 500 to 1000 psi, difficult ST removal after
	SS-4	75	17-37-43- 41		Gradual layer transition. Sandy Silt (ML)- About 61% non-plastic fines; about 39% fine to medium sand;				difficult ST removal after pushing.
5	ST-3	100			53% fines; 47% fine to medium sand; trace of gravel; light brown.	NV	NP	9	grass vegetation ground cover. ST-3; crowd pressures from 300 to 1000 psi,
	SS-2	71	10-10-10-9	•	Light particle cementation noted from 2.5' to 4.5'.				Driller: Haz-Tech Drilling Boring located on relativ flat terrain with about 50
	SS-1	46	1-4-6-12		Sandy Silt (ML)- (NATIVE)				CME-75; Automatic Hammer SPT per ASTM D1586; NWJ rods;
TYPE - No.	TYPE - No.	RECOVERY 9 (RQD)	BLOW COUNTS	ГІТНОГОGY	MATERIAL DESCRIPTION (Stratification lines represent approximate boundaries between materials)	(%) TT	PI (%)	MC (%)	REMARKS
		%		~		LA	B DA	λTA	





Photo 1: Boring B-1, sample SS-2 at 2.5 feet Sandy Silt (ML)



Photo 2: Boring B-1, sample SS-6 at 12.5 feet Sandy Silt (ML)





Photo 3: Boring B-1, Rock Coring from 20.0 to 29.5 feet Basalt Rock



Photo 4: Boring B-1, Rock Coring from 29.5 to 31.5 feet Basalt Rock

#### PROJECT: Mountain Home Landfill LOCATION: Elmore County, Idaho

#### METHOD: Hollow-Stem Auger DATE LOGGED: 11/12/2015 LOGGED BY: Ryan VanLeuven, PE

LONGITUDE: -115.575514° by Client

LATITUDE: 43.075839°

**BORING NO. B-2** 

#### MERICAN A S CHNIC E

#### **GROUNDWATER:**

Groundwater not encountered on 11/12/2015

DEPTH (ft)	ТҮРЕ - No.	ТҮРЕ - No.	RECOVERY % (RQD)	BLOW COUNTS	ГІТНОГОЄУ	MATERIAL DESCRIPTION (Stratification lines represent approximate boundaries between materials)	REMARKS
		SS-13	54	1-7-16-15	••••	Sandy Silt (ML)- (NATIVE) About 50% non-plastic fines; about 50% fine to medium sand; moist; light brown.	CME-75; Automatic Hammer SPT per ASTM D1586; NWJ rods;
		SS-14	54	17-15-11-6		Light particle cementation noted from 4.0' to 4.5'.	Driller: Haz-Tech Drilling Boring located on relatively flat terrain with about 20% grace vegetation ground
- 5		ST-15	100		•		grass vegetation ground cover. ST-15; crowd pressures from 400 to 950 psi.
-				23-35-33-		Gradual layer transition.	
		SS-16	71	38		Sandy Silt (ML)- About 60% non-plastic fines; about 40% fine to medium sand; slightly moist; light brown.	
- 10		ST-17	100		•	Moderate particle cementation noted from 8.0' to 9.5'.	ST-17; crowd pressures from 650 to 1000 psi.
-		SS-18	78	19-26-50/6"	· .	Light particle cementation noted from 12.5' to 14.5'.	
- 15		ST-19	100				ST-19; crowd pressures from 400 to 1000 psi.
-		SS-20	83	6-6-38-40			
- 20		SS-21	67	14-43-50/6"		Sandy Silt with Gravel (ML)- About 40% non-plastic fines; about 40% fine to coarse, subangular to angular sand; about 20% moderately hard, angular basalt gravel	Slight auger grinding from 19.4' to 22.5'.
						to 1"; slightly moist; brown.	SS-22; SS sampler bouncing on rock.
-	<u>SS-22</u>	CR-23	100			About 80% fine to coarse, subangular to angular sand; about 20% non-plastic fines; light particle cementation; slightly moist; tan. Basalt Rock	Switch to HQ core at 22.5'.
- 25		UR-23	(63)			Highly fragmented from 22.5' to 23.0'. Dark gray, moderately hard, medium vesicularity, low to moderately	
-						high discontinuity angles, moderately rough to rough joint faces, moderately spaced fractures. Possible rouble zone or void from 27.0' to 27.5', little recovery.	
- 30		CR-24	88 (56)			As above, except: widely spaced fractures from 28.5' to 32.0', low vesicularity from 30.2' to 33.5'.	
-		CR-25	100 (80)			As above, except: medium vesicularity from 33.5' to 36.0', moderately spaced fractures from 32.0' to 36.0', low to vertical discontinuity angles.	

GROUNDWATER: Groundwater not encountered on	
Groundwater not encountered on	
	TECHNICS
MATERIAL DESCRIPTION (Stratification lines represent approximate boundaries between materials)	REMARKS
3	(Stratification lines represent approximate boundaries between materials)





Photo 1: Boring B-2, sample SS-14 at 2.5 feet Sandy Silt (ML)



Photo 2: Boring B-2, sample SS-18 at 12.5 feet Sandy Silt (ML)





Photo 3: Boring B-2, Rock Coring from 22.5 to 33.3 feet Basalt Rock



Photo 4: Boring B-2, Rock Coring from 33.3 to 36.0 feet Basalt Rock

## PROJECT: Mountain Home Landfill LOCATION: Elmore County, Idaho

#### METHOD: Hollow-Stem Auger DATE LOGGED: 11/12/2015 LOGGED BY: Ryan VanLeuven, PE

**BORING NO. B-3** 

#### 

#### **GROUNDWATER:**

Groundwater not encountered on 11/12/2015

	43.075 <sup>,</sup> E: -115.	06° <sup>by Clie</sup>	nt

DEPTH (ft)	TYPE - No.	TYPE - No.	RECOVERY % (RQD)	BLOW COUNTS	ГІТНОГОGY	MATERIAL DESCRIPTION (Stratification lines represent approximate boundaries between materials)	REMARKS
_		SS-26	63	1-1-1-1		Sandy Silt (ML)- (NATIVE) About 50% non-plastic fines; about 50% fine to medium sand; moist; light brown.	CME-75; Automatic Hammer SPT per ASTM D1586; NWJ rods; Driller: Haz-Tech Drilling
- 5		SS-27 ST-28	75	6-12-30-24		Gradual layer transition. Sandy Silt (ML)- About 60% non-plastic fines; about 40% fine to medium sand; slightly moist; light brown.	Boring located on relatively flat terrain with about 15% grass vegetation ground cover. ST-28; crowd pressures to
- - 10		SS-29	92	16-23-28- 29	·	Very light particle cementation noted from 7.5' to 9.5'.	1000 psi.
		ST-30	100	8-16-15-		Moderate particle cementation noted from 13.2' to 14.2'.	ST-29; crowd pressures from 750 to 1000 psi. SS-31; basalt gravel in tip
- 15		SS-31 OSS-32	75 100	8-16-15- 25/2" 11-16-17		Silty Sand with Gravel (SM)- About 60% fine to coarse, subangular to angular sand; about 20% non-plastic fines; about 20% moderately hard, subangular to angular basalt gravel to 1.5"; slightly moist; tan.	of SS sampler, SS bouncing during driving. OSS-32; 2 rings recovered.
-	<u>SS-33</u>	CR-34	100 (0)			Basalt Rock- Dark gray, moderately hard, medium vesicularity, high to vertical discontinuity angles, moderately rough to rough joint faces, very	SS-33; SS sampler bouncing on rock, basalt in tip of sampler.
- 20		CR-35	100 (43)			closely spaced fractures. As above, except: closely to widely spaced fractures.	Switch to HQ core at 17.5'.
- 25		CR-36	100 (85)			As above, except: low vesicularity.	
	<u> </u>	<u> </u>	<u> </u>			Bottom of Boring at 29.5 ft on 11/12/2015.	Backfilled hole with bentonite chips.
FILE	NO. 0	2786				PAGE 1 OF 1	BORING NO. B-3





Photo 1: Boring B-3, sample SS-26 at 0.0 feet Sandy Silt (ML)



Photo 2: Boring B-3, sample SS-29 at 7.5 feet Sandy Silt (ML)





Photo 3: Boring B-3, Rock Coring from 17.5 to 26.5 feet Basalt Rock



Photo 4: Boring B-3, Rock Coring from 26.5 to 29.5 feet Basalt Rock

#### PROJECT: Mountain Home Landfill LOCATION: Elmore County, Idaho

#### METHOD: Hollow-Stem Auger DATE LOGGED: 11/13/2015 LOGGED BY: Ryan VanLeuven, PE

**BORING NO. B-4** 

#### AMERICAN TECHNICS

#### **GROUNDWATER:**

LAT LON	itude: Igitudi	43.07 E: -11	4458° 5.575511°	by Clie	Groundwater not encountered on 11/13/2015	ECHNICS
DEPTH (ft)	TYPE - No.	RECOVERY % (RQD)	BLOW COUNTS	ГІТНОГОСУ	MATERIAL DESCRIPTION (Stratification lines represent approximate boundaries between materials)	REMARKS
	SS-37	71	2-2-9-22		Sandy Silt (ML)- (NATIVE) About 50% non-plastic fines; about 50% fine to medium sand; moist; light brown.	CME-75; Automatic Hammer SPT per ASTM D1586; NWJ rods; Driller: Haz-Tech Drilling
5	SS-38 ST-39	54 100	11-15-13- 16	·	Gradual layer transition. Sandy Silt (ML)- About 60% non-plastic fines; about 40% fine to medium sand; slightly moist;	Boring located on relatively flat terrain with about 30% grass vegetation ground cover.
	SS-40	94	9-18-36		light brown.	ST-39; crowd pressures from 700 to 1000 psi. OSS-40; 2 rings recovered.
10	<u>ST-41</u>	100				ST-41; crowd pressures to 1000 psi. OSS-42; 2 rings
15	OSS-42 SS-43	82 67	13-30-50/5" 31-50/6"		trace of basalt gravel from 15.9' to 16.0'.	recovered. SS-43; SS sampler
20	CR-44	100 (93)			Basalt Rock- Dark gray, moderately hard, medium vesicularity from 16.0' to 25.2' then low vesicularity from 18.5' to 25.2', horizontal to moderately high discontinuity angles, moderately rough to rough joint faces, moderately spaced fractures.	- bouncing on rock. Switch to HQ core at 16.0'
25	CR-45	100 (82)			As above, except: medium vesicularity from 25.2' to 26.0'.	
					Bottom of Boring at 26.0 ft on 11/13/2015.	Backfilled hole with bentonite chips.
ILE	E NO. 02	2786			PAGE 1 OF 1	BORING NO. B-4





Photo 1: Boring B-4, sample SS-38 at 2.5 feet Sandy Silt (ML)



Photo 2: Boring B-4, sample SS-43 at 15.0 feet Sandy Silt (ML)





Photo 3: Boring B-4, Rock Coring from 16.0 to 26.0 feet Basalt Rock

#### PROJECT: Mountain Home Landfill LOCATION: Elmore County, Idaho

## METHOD: Hollow-Stem Auger DATE LOGGED: 11/13/2015

LOGGED BY: Ryan VanLeuven, PE

#### **BORING NO. B-5**



#### **GROUNDWATER:**

Groundwater not encountered on 11/13/2015

LAT LON	TUDE: GITUDE	43.07: E: -11:	3778° 5.574403°	by Clie	Groundwater not encountered on 11/13/2015	ECHNICS
DEPTH (ft)		RECOVERY % (RQD)	BLOW COUNTS	ГІТНОГОЄУ	MATERIAL DESCRIPTION (Stratification lines represent approximate boundaries between materials)	REMARKS
_	SS-46	63	1-4-6-9	-•••••	Sandy Silt (ML)- (NATIVE) About 50% non-plastic fines; about 50% fine to medium sand; moist; light brown.	CME-75; Automatic Hammer SPT per ASTM D1586; NWJ rods; Driller: Haz-Tech Drilling
- 5	OSS-47	89	12-17-23	-•••••		Boring located on relatively flat terrain with about 40% grass vegetation ground
_	ST-48	100				cover. OSS-47; 2 rings recovered.
- 10	<u>SS-49</u> CR-50	100 100 (38)			Poorly Graded Sand with Silt and Gravel (SP-SM)- About 60% fine to coarse, subangular to angular sand; about 30% medium hard, subangular to angular basalt gravel to 1"; about 10% non-plastic fines; slightly moist; tan to gray.	ST-48; crowd pressures from 600 to 1000 psi. Switch to HQ core at 8.5'.
- 15	CR-51	100 (95)			Basalt Rock- Some soil infilling of fractures from 9.8' to 10.5'. Dark gray, moderately hard, medium vesicularity except low vesicularity from 12.0" to 14.5', horizontal to low discontinuity angles, moderately rough to rough joint faces, moderately spaced fractures. Some soil infilling of fractures from 12.0' to 17.5'.	
_	CR-52	100 (63)			As above, except: low to vertical discontinuity angles; widely spaced fractures.	
					Bottom of Boring at 19.5 ft on 11/13/2015.	Backfilled hole with bentonite chips.





Photo 1: Boring B-5, sample SS-46 at 0.0 feet Sandy Silt (ML)



Photo 2: Boring B-5, sample SS-49 at 7.5 feet Poorly Graded Sand with Silt and Gravel (SP-SM)





Photo 3: Boring B-5, Rock Coring from 8.5 to 17.5 feet Basalt Rock



Photo 4: Boring B-5, Rock Coring from 17.5 to 19.5 feet Basalt Rock

			ntain Home Landfill ore County, Idaho		Т	ES	T PIT NO. TP-1
MET DAT LOG	HOD: E LOG GED B	John I GED: Y: Ry	Deere 310 SG 4/11/2016 an VanLeuven, PE GROUNDWATER: Groundwater not encountered on 4/11/2016 5.573312808° by Hand Held GPS			A	
LON	GITUD	E: -11:	5.573312808°	1.4	B DA	ΔΤΔ	
DEPTH (ft)	TYPE - No.	ГІТНОГОGY	MATERIAL DESCRIPTION (Stratification lines represent approximate boundaries between materials)	TT (%)	PI (%) IA	MC (%)	REMARKS
- 5	BK-58		Lean Clay (CL)- 93% fines; 7% fine to medium sand; dark brown. Lean Clay with Sand (ML)-	46	29	13	Very stiff digging from about 2.0' to 7.5. Stiff digging from about 7.5' to BOH.
- 10	BK-59		About 80% non-plastic fines; about 20% fine to medium sand; trace of gravel; slightly moist; light brown. Bottom of Test Pit at 12.0 ft on 4/11/2016.				7.5' to BOH.
		2796	PAGE 1 OF 1				TEST PIT NO TP-1

PRC	JECT:	Mour I: Elm	ntain Home Landfill ore County, Idaho		Т	ES	T PIT NO. TP-2
MET DAT	'HOD: E LOG	John iGED:	Deere 310 SG 4/11/2016 <i>r</i> an VanLeuven, PE <u>GROUNDWATER:</u>			A	
LAT LON	ITUDE IGITUE	: 43.07 )E: -11	Groundwater not encountered on 4/11/2016 5.572458524° by Hand Held GPS			T	ECHNICS
t)	o.	Ϋ́		LA	B D#	ATA	-
DEPTH (ft)	TYPE - No.	ГІТНОГОСУ	MATERIAL DESCRIPTION (Stratification lines represent approximate boundaries between materials)	(%) TT	PI (%)	MC (%)	REMARKS
-			Silty Clay with Sand (CL-ML)- 78% fines;				Stiff digging from about 2.0' to BOH.
- 5	BK-56		22% fine to medium sand; light brown.	27	6	13	
- - 10							
	BK-57		Bottom of Test Pit at 12.0 ft on 4/11/2016.				
<b>EU E</b>	: NO (	12786	PAGE 1 OF 1				TEST PIT NO TP-2

DAT LOG	e logo Ged B'	GED: Y: Ry	Deere 310 SG 4/11/2016 van VanLeuven, PE 5944772° 5.573253045° by Hand Held GPS				
DEPTH (ft)	TYPE - No.	ГІТНОГОЄУ	MATERIAL DESCRIPTION (Stratification lines represent approximate boundaries between materials)	LAI (%) TT	B D/	MC (%)	REMARKS
- 5 - 10	BK-54 BK-55		Silt with Sand (ML)- 85% fines; 15% fine to medium sand; light brown.	26	4	8	Stiff digging from about 2.0' to BOH.
			Bottom of Test Pit at 12.0 ft on 4/11/2016.				

PROJECT: Mountain Home Landfill LOCATION: Elmore County, Idaho

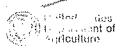
**TEST PIT NO. TP-3** 

		ntain Homo				WE	LL NO. B-6
ATE LOG	GED:	w-Stem Au 4/11/2016 /an VanLei	-		PE <u>GROUNDWATER:</u>		
ATITUDE: ONGITUD	43.07 E: -11	514254° 5.5762755	59°	, by	Groundwater not encountered on 4/		HNICS
TYPE - No.	RECOVERY % (RQD)	BLOW COUNTS		<b>ГПНОГОGY</b>	MATERIAL DESCRIPTION (Stratification lines represent approximate boundaries between materials)	REMARKS	WELL DIAGRAM
SS-60	67	6-6-4		•	Sandy Silt (ML)- (NATIVE) About 50% non-plastic fines; about 50% fine to medium sand; trace of gravel; light brown.	CME-85; Automatic Hammer SPT per ASTM D1586; NWJ rods; Driller: Haz-Tech Drilling	About 3' of pipe rise above ground surface,
5 SS-61		4-5-13		•   •   •   •	Light particle cementation noted from 5.0' to 6.5'.	Boring located on relatively flat terrain with about 50% grass vegetation ground cover.	<ul> <li>encased in locked vault.</li> <li>Concrete from ground</li> </ul>
SS-62	89	9-14-13	-    -    -    -			GW reading on 5/11/2016: no measurable GW present in observation well.	surface to about 3.0'
0 SS-63	100	18-17-14			Gradual layer transition. Sandy Silt (ML)- About 60% non-plastic fines; about 40% fine to medium sand; trace of gravel; slightly moist; light	-	
SS-64	89	3-13-16		•	brown. Moderate particle cementation noted from 10.0' to 11.5'.		
5 SS-65	100	18-21-29				SS-65; small basalt gravel in tip of SS sampler.	
					Basalt Rock-	Hard auger grinding and slow drilling from 17.0' to 20.0'.	
CR-66	100 (90)				Dark gray, moderately hard, low to medium vesicularity, horizontal to moderately high discontinuity angles, moderately rough to rough joint faces, closely to moderately spaced fractures.	Switch to HQ coring at 20.0'. Core return water gray from 20.0' to 36.0'. No core water return 36.0' to 60.0'.	■Bentonite Chips
5 CR-67	100 (77)				Dark gray except red-gray from 27.0' to 30.0', moderately hard, low to medium vesicularity, horizontal to low discontinuity angles, moderately rough to rough joint faces, very close to closely spaced fractures. Highly fractured from 27.5' to 28.0' and 29.0' to 29.5'.		
0 CR-68	100 (67)				Dark gray to red-gray, moderately hard, medium vesicularity, horizontal to high discontinuity angles, moderately rough to rough joint faces, very close to closely spaced fractures. Highly fractured from 32.5' to 34.0'.		
LE NO. 0	2796		Ëll		PAGE 1 OF 2		WELL NO. B-6

PAGE 1 OF 2

		tain Hom ore Count			W	ELL NO. B-6
				Groundwater not encou	Intered on 4/11/2016	<b>EEO</b>
TYPE - No.	RECOVERY % (RQD)	BLOW COUNTS	LITHOLOGY	MATERIAL DESCRIPTION (Stratification lines represent approximate boundaries between materials)	REMARKS	WELL DIAGRA
CR-69	100 (77)			Dark gray, moderately hard, medium vesicularity, horizontal to high discontinuity angles, moderately rough to rough joint faces, very close to closely spaced fractures.		
CR-70	100 (55)			Dark gray, moderately hard, low to medium vesicularity, horizontal to high discontinuity angles, moderately rough to rough joint faces, very close to closely spaced fractures.		
CR-71	87 (60)			Dark gray except red-gray from 49.0' to 50.0', moderately hard, low to medium vesicularity, horizontal to high discontinuity angles, moderately rough to rough joint faces, very close to moderately spaced fractures. Driller notes void or cinders from 49.0' to 50.0'.		
CR-72	80 (23)			Driller notes voids or cinders from 53.5' to 60.0', little recovery.		Slotted pipe backfille with san from 40. to 60.0'
CR-73	60 (0)					Cap on
5				Bottom of Well at 60.0 ft on 4/11/2016.		bottom o pipe
LE NO. 0	02786			PAGE 2 OF 2		WELL NO. B-6

#### Appendix 6



\*\*\*\*\*

Soil Conservation Service 3160 Elder Street Suite A Boise, Idaho 83705

Sublect: Bennett Rd. Landfill Site

Date: March 23, 1988

File code: 430 +

<sup>To:</sup>Roy Fowler, DC Mountain Home FO Idaho

On March 22, 1988 we made deep test hole examination of the proposed Elmore County Sanitary Landfill along Bennett Road just northeast of Mountain Home. Attached is a sketch map of the area showing approximate property lines and rough locations of the test pits.

Generally the test holes showed no restrictive or limiting layers for landfill use except as stated below. These pits were dug to depths of 18 to 20 plus feet. The following excavations showed limiting layers:

- Test pit #3- A strong inducated duripan was incurred at a depth of 18 inches and continued to more than 5 feet. This material was extremely hard to dig and would be a problem in operation of the landfill. Digging was not continued below 7 feet.
- Test pit #4- Bedrock was encountered in this pit at a depth of 7'6". Digging was discontinued. Another test pit about 100 feet northeast, pit #5, showed no bedrock within 20 feet in depth.

As a result of the observation of these test pits I conclude that this to be a suitable landfill site from the standpoint of soil conditions from Bennett Road south to where the power line crosses the property. There are no visible signs of high water table or other limiting features in the soil profiles.

I have more detailed profile information on the test pits should you need it.

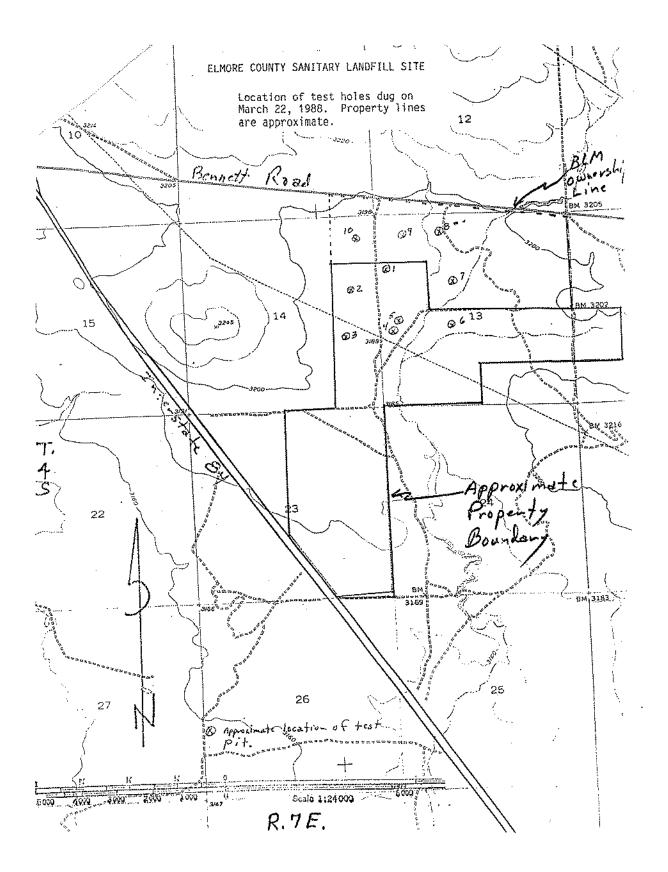
Harley R. Noe Area Soil Scientist

attach

The Soil Conservation Service is an agency of the United States Department of Agriculture



g U.S. Government Printing Office: (355-325-366/30677



307313 Form 238-7 046 075 144441 STATE O	)F ID/	ано			USE TYPEWAII		R
6/89 U45-0/E-14AAA1 DEPARTMENT OF WATER RESOURCES BALLPOINT PEN							
State law requires that this report be filed with the Director. Department of Water Resources							
Department of 20-mit Resources within 30 days after the comple	tion or	aband	onmen	t of the well.	·		
1. WELL OWNER	7.	WATE	R LEV	/EL			
Name Elmore County Land Pill	Static water level <u>14</u> feet below land surface. Flowing? □ Yes □ No   G.P.M. flow						
Audress 1605 3rd East mt Hom	Artesian closed-in pressure p.s.i. Controlled by: □ Valve □ Cap □ Plug						
Owner's Permit No. <u>61-89-2-020</u>	Temperature PF. Quality Describe artesian or temperature zones below.						
2. NATURE OF WORK	8. WELL TEST DATA						
I New well  □ Deepened  □ Replacement □ Well diameter increase	□ Pump □ Bailer Ø Air □ Other						
Abandoned (describe abandonment procedures such as materials, plug depths, etc. in lithologic log)		Discharg	с G.P.M С	Hours P	Hours Putoped		
3. PROPOSED USE							
					02203	Q	
Domestic Infigation Test Information Industrial Stock Waste Disposal or Injection	9. Bore	LITH			·····	····	iter
Other (spacify type)	Diam.	From	То	Materi	al		s No
4. METHOD DRILLED	10" 10"	0 3	3 _೨೭	Jop soil soll w/ cla	····		Ŕ
🗵 Rotary 🔟 Air 🔲 Hydraulic 🗂 Reverse rotary	10"		40	gray lova + c	inder Morok	n)	Ŕ
Cable Dug Other	6	<i>1</i> 05	67	Fractured la	<u>va</u>		X
5. WELL CONSTRUCTION	6	. (a'T.	610	(Last returning gray lave)		<u>}</u>	Ŕ
Casing schedule: Z Steel Concrete Other Thickness Dlameter From To			•				
.250 inches 8 1/8 inches + 1 feet 47 feet							
inches inches feetfee		······································					
inches inches feet feet feet			<u>.</u> .				
Was a packer or seal used? 🗀 Yes 🛛 No				D			+
Perforated?				N/2			
Size of perforation inches by inches inches To				SEP 26 19			
perforations feet feet		,,		······································			
perforations feet feet				Depattment of Water	Resources	-	
Weil screen installed? 🔲 Yes 🛛 Ko Manufacturer's name				ين ورايين ويد	2.72.775.2775.5		
Type Model No Diameter Slot sizeSet fromfeet tofeet			·				
DiameterSlot sizeSet fromfeet tofeet Gravel packed?				- Hill Comme	<u></u>	_	
Placed from feet to feet					999	-	-
Surface seal depth <u>4</u> Material used in seal: Cement grout Bentonite Duddling clay	6		AFH	A 1 Construct of Water of Western Regiona	Resources		
Sealing procedure used:		8 <b>- 2</b> - 2 - 2	64.61				<u>†</u>
Method of joining casing:  Threaded  Welded  Solvent Weld		JUN	13	990	····		÷
Cemented between strata	10						<u> </u>
Describe access port	10.	Wo	rk start	ted Aug 1, 1989 fini	ished Aug Ke,	1489	ì
6. LOCATION OF WELL	11,			CERTIFICATION		OI	
Sketch map location <u>must</u> agree with written location, N				that all minimum well co b at the time the tin was re		ards w	ere
Subdivision Name		complied with at the time the rig was removed. Firm Name <u>Hiddleslap Oniting</u> Firm No. <u>35</u>					
W E					•		
Lot No Block No	Address <u>Mt. Home Id</u> Date <u>Aug 21, 1989</u> Signed by (Firm Official)						
S S		Signed	by (Fi	and	STILLA	lice	47
			t	Operator)	SKida	las	K
SE & NE & Sec. 14 . T. 1 SZR. 7 W							

USE ADDITIONAL SHEETS IF NECESSARY - FORWARD THE WHITE COPY TO THE DEPARTMENT

393132							
04S-07F-13AAD1	UU MAR 6 1967						
State o:			Department or Reclamation				
State law requires that this report shall	l be :	filed					
Engineer within 30 days after completion or a	bandoi	ament	of the well.				
Reme I NITIX VIA LYLALIZIO	Jdenth	101	well: Standing water				
Address	level Fahr.	l bel	ow ground: 199 Temp." "Test delivery: 30 gpm				
Owner's Permit No.	or cfs Pump? [7] Bail Size of pump and motor used to make test:						
NATURE OF WORK (check): Replacement well							
New well Deepened Abandoned	Leng	Length of time of test:HrsMin Drawdown:ft. Artesian pressure: ft.					
Water is to be used for:	above	bove land surface Give flow cfs r gpm. Shutoff pressure:					
Dug Other	Conti	colle	d by: Valve Cap Plug				
(explain) CASING SCHEDULE: Threaded Velded	Yes	$\square$	Does well leak around casing?				
"Diam. from $\beta$ ft. to $26$ ft. "Diam. from ft. to ft.	THE REAL	<b>PTH</b>	MATERIAL 04300 WATER				
	TEET	FEET	Seva wik				
"Diam. from ft. to ft. Thickness of casing: Material:	440	509	Broken Java andera				
Steel concrete wood other	504	435 435	Some Small cinders				
	<u> </u>						
(explain) PERFORATED? Yes No Type of	L		Hole drilled to 635				
perforator used:			Then reamed to				
Size of perforations:" by"			16 8-304				
perforations from ft. to ft.			Rock from 440 to 504				
perforations fromft. toft. perforations fromft. toft.			was very broken.				
perforations from ft. to ft. WAS SCREEN INSTALLED? Yes No			Brock from about 4 30				
Manufacturer's name			to- 130 4 Decy hard				
Type     Model No.       Diam.     Slot size     Set from     ft. to     ft.       Diam.     Slot size     Set from     ft. to     ft.							
CONSTRUCTION: Well gravel packed? Yes No. size of gravel Gravel							
placed from ft. to ft. Surface seal			· · · · · · · · · · · · · · · · · · ·				
provided? Yes No To what depth? ft. Material used in seal:							
Did any strata contain unusable water? Yes							
No. Type of water:							
Depth of strata ft. Method of sealing strata off:	<b></b>						
Surface casing used? Yes No.							
Cemented in place? Yes. No							
Locate well in section							
			ted:				
			.shed:				
Sec.	drilled under my supervision and this report						
	ls ti Name	.ue (	Wayne Steven				
	Addre	ess:_	3704 Hawtherne Bain				
	Signe	d by					
LOCATION OF WELL: County Climon		ise N	lo Date:				
NEX NEX Sec. 13 T. 45 N/S R. 7E E/WRSE							
Use other side for	addit	iona	l remarks				

USGS

136 30 アマック 889 3.0 - pupeter 401280 04S-07E-13AAD2 REPORT OF WELL DRILLER State of Idaho State law requires that this report shall be filed with the State Reclanat Engineer within 30 days after completion or abandonment of the well. WELL OWNER Size of drilled hole: Total depth of well: <u>1840</u> Standing water level below ground: <u>477</u> Temp. Fahr.\_\_\_\_\_° Test <u>delivery</u>:\_\_\_\_\_ Name Address \_gpa or\_\_\_\_ cfs Pump? Bail Owner's Permit No.\_\_ Size of pump and motor used to make test: 678326 NATURE OF WORK (check): Replacement well noted New well X Deepened Abandoned Length of time of test: Hrs. Min Drawdown: ft. Artesian pressure: ft. Hrs. Min. Water is to be used for: above land surface Give flow cfs or gpm. Shutoff pressure: Controlled by: Valve Gap Plug No control Does well leak around casing? Yes No X METHOD OF CONSTRUCTION: Rotary Cable Dug 🗌 Other (explain) CASING SCHEDULE: Threaded \_\_\_\_\_ Welded 30 "Diam. from 10" "Diam. from ft. ft. to 41 DEPTH 0 MATERIAL 104299ATER NO  $\frac{1}{26} \frac{1}{2} \frac{1$ FROM TO FEET FEET 100 0 no 19 217 244 other 🗌 no 245 no 330 366 222 (explain) 363 41 no PERFORATED? Yes No X Type of 417. 10 perforator used: 540 apeg たぶれ hor Size of perforations: # by nt perforations from ft. to ft. na ſt. MAGNET 12 perforations from ft. to 1272 13.50 nc perforations from THUGHER TO A ft. to ft. 1350 135 ni perforations from ft. to ft. 1355 1840 no WAS-SCREEN INSTALLED? Yes No Manufacturer's name Type NEAR THE BOTTOM OF THE Model No. \_\_\_\_\_\_Slot\_size\_\_\_\_Set\_from\_\_\_ft.to\_\_\_\_\_ \_\_\_\_\_Slot\_size\_\_\_\_Set\_from\_\_\_ft.to\_\_\_\_ Diam. ft WELL HOT MUD WAS Diam. ft. ENCLUNTORED WITH VORY CONST<u>RUCTION: Well gravel packed? Yes</u> LITLE WATER No. X size of gravel Gravel placed from ft. to ft. Surface ses provided? Yes No To what depth? \_\_\_\_\_ft. Material used in seal: Gravel Surface seal Did any strata contain unusable water? Yes \_\_\_\_\_ No. X Type of water:\_\_\_\_\_\_ Depth of strata ft. Method of sealing strata off: Surface casing used? Yes X No. No X Locate well in section Work started: Man 1967 Work finished: Same sul Well Driller's Statement: This well was Seca drilled under my supervision and this report 13 is true to the trait of my knowledge. Address: 📈 Signed by: License No. 133 Date: ///au LOCATION OF WELL: County NE \* NE \* Sec. 13 T. 4-5 NS R. 7 E/ Use other side for additional remarks t the second second serviced 1 N. 2012

61

· · · ·	G RECEIVED					
422807						
04S-07E-13AAD3 REPORT OF WI State o:	MAR 24 1969					
State of	r Idaho Department of Reclamation					
State law reduires that the report shall be filed with the State Reclamation Ingineer within 30 days moder completion or abandonment of the well. Size of drilled hole:						
IELL OWNER: ASTANISMENT	Size of drilled hole: <u>6</u> Total depth of well: <u>2045</u> Standing water					
iddress	Devel below mound it a Pewn.					
	Fahr. Test delivery: gpm or cfs Pump? Bail					
Wher's Permit No. 9-1 G. 2832( NATURE OF WORK (check): Replacement well	Size of pump and motor used to make test:					
lew well Deepened L Abandoned	Length of time of test: <u>ALANEH</u> rs. Min. Drawdown:ft. Artesian pressure: ft.					
Ater is to be used for:	above land surface Give flow cfs					
bug Other Other	Controlled by: Valve Cap Plug No control Does well leak around casing?					
ASING SCHEDULE: Threaded Welded Welded 6 "Diam. from 7 2 ft. to - 1900 ft.	Yes No MATERIAL WATER					
"Diam. from ft. to ft.	FROM TO DES OR NO					
"Diam, from ft. to ft.	13/5 1200 cleanant class a hitrate					
Thickness of casing: 1/4 " Material:	1900 1905 flack rock					
iteel concrete wood coher	20134 2045 Lange Jack					
(explain)						
PERFORATED? Yes No Lype of perforator used:						
Size of perforations: " by "						
periorations from the to the						
perforations fromft. toft. perforations fromft. toft.						
perforations fromft. toft.						
Manufacturer's name Type Model No						
Diam. Slot size Set from ft. to ft Diam. Slot size Set from ft. to ft						
CONSTRUCTION: Well gravel packed? Yes						
No size of gravel Gravel placed fromft. toft. Surface seal						
provided? Yes No To what depth? ft. Material used in seal:						
Did any strata contain unusable water? Yes						
No. [2] Type of water: Depth of strataft. Method of sealin strata off:						
<u> </u>						
Surface casing used? Yes No.						
Locate well in section						
	Work started: Day 10 - 69					
	Work finished Test 20-69 Well Driller's Statement: This well was					
Sec	drilled under my supervision and this report is true to the best of my knowledge.					
· · · · · · · · · · · · · · · · · · ·	Name: Name: I and a man and a start and					
	Address: Bay 993 Mandell, dolaho Signed by: Dale C. Dilbert					
	License No. 17 Date: march 4-69					
LOCATION OF WELL: County Elmole NE X NE Sec. 13 T. 4 8/S R. 7 E/S	- Jonn					
Use other side for additional remarks						
61	OVER USGS					

I Did this Aulling for m. Dean Rogers sooking, Allaho and he said that remit was in charly House' nome and that you had permit no. & location of well on file

This well is located openant. 7 min South East of mit. Home on remnet road.

The 6" pipe has a thick fitnite cel from 13.65 to 1900 -

Deor mr. Johnson: Inregards to your letter dated morch 24, 1969. I'm sorry fout not putting enough impormation on log but will try to give you impormation you want. 1= O didn't spirt thermometer in cuttings recovered by bales but by feeling by hand the clay was oppray 110° to 120° the lock was road. 2 = To my knowledge Ockink mr. Rogers is undecided at this time 3 = There was a steel plate welded on top of careing whend left well 4 - I don't believe notion well is seepege, fecause after running 6" pipe I filled well to top with water and replaced it as I Drilled and boled and at ofour 1925 water level dropped to 427 and a couldn't fill it fook up by howling moty lith truck and 1000 Jollon water tank. Under ordenary Drilling and foleong SWL remained at 427 fx Ofole Duell every 5or 6 for with 44 "1. O foler 40 for long. If you meed any further impormation please write me

• . . . . . .

Very Truly yours, Splet Selfert



REPORT OF WELL DRILLER State of Idaho



State law requires that this report shall be filed with the State Reclamation Engineer within 30 days after completion or abandonment of the well.

WELL OWNER: Name CHAS, R. ROUSE	Size of drilled hole: <u>0.7/8</u> " Total depth of well: <u>811</u> Standing water	
Address 2523 Inglewood Brad.	level below ground: Temp. Fahr ° Test delivery: or cfs Pump? [ Bail	
	Fahr. <sup>°</sup> Test delivery:	_gpa
Owner's Permit No. <u>G-28326</u>	Size of pump and motor used to make tes	st:
NATURE OF WORK (check): Replacement well		
New well X Deepened Abandoned	Length of time of test: Hrs. M	lin.
Water is to be used for: Irrigation	Drawdown: ft. Artesian pressure: f	īt.
METHOD OF CONSTRUCTION: Rotary X Cable	above land surface Give flow cf or gpm. Shutoff pressure:	6
	Controlled by: Valve Cap Plug	<del></del>
(Awalata)	No contral Door will look second to	
CASING SCHEDULE: Threaded Welded	Yes No No MATERIAL ()4302 YES	-
"Diam. from ft. to ft. "Diam. from ft. to ft. "Diam. from ft. to ft. "Diam. from ft. to ft. "Diam. from ft. to ft. Thickness of casing:Material:	DEPTH MATERIAL 04302	ATER
"Diam from It. to It.	FROM TO TES	ORN
"Diam. from ft. to ft.		.No.
Thickness of casing: Material:	0 2 Soil, dark 2 6 Clay, vellow 6 8 Clay, yellow, hard	no
Steel _ concrete _ wood _ other _	5 8 Clay, yellow, hard	no
Recer [] concrete [] mood [] concret	b of to band, black	no.
	10 14 Clay, brown	no
(explain) PERFORATED? Yes No Type of	14 18 Sand, brown, fine 18 22 Clay, vellow	no
perforator used: No Type of		_no
	15 19 Lava, black with red streeks	no no
Size of perforations: "by "	49 65 Lava, black 65 73 Lava, red and brown	10
Size of perforations: "by " perforations fromft. toft.	65 73 Lava, red and brown	ne.
ft. toft.	73 31 Lava	no
perforations fromft. toft. perforations fromft. toft. perforations fromft. toft.		
WAS SCREEN INSTALLED? Yes No	This well started with a 9-7/8" test hole. Lost circulation	
Manufacturer's name	several times starting at 291.	
Manufacturer's name TypeModel No DiamSlot sizeSet fromft. toft	but was able to regain circulat	Lon
Diam. Slot size Set from ft. to ft	until Sl' was reached. Lost	
DiamSlot sizeSet fromft. toft	circulation at this point and	
CONSTRUCTION: Well gravel packed? Yes 🗌	Was never able to regain it.	
No size of gravel Gravel placed fromft. to ft. Surface seal	Agreement made with customer to	
placed from it. to ft. Surface seal	going to get a cable tool wing	
provided? Yes No To what depth? ft. Material used in seal:	rig on the job.	
IV. MUUUIAA ESCI IN SEAL.		
Did any strata contain unusable water? Yes		· · · · ·
No. [] Type of water: Depth of strataft. Method of sealing		
Depth of strataft. Method of sealing	<b>****</b>	
strata off:		
Surface casing used? Yes No.		
Cemented in place? Yes No		
Locate well in section		
		··-
x		
	Work started: Jan 12, 1065 Work finished: Feb. 2, 1965	
	Well Driller's Statement: This well was	
	drilled under my supervision and this r	
	is true to the best of my knowledge.	-
	Name: Levi Jorgensen for B. & M. Souin	. <u>Co.</u>
	Address: p.C. Fox 220, Celdwell, Idaho	
	Signed by the spream	
	License No. 221 Date: Feb. 5, 1965	
LOCATION OF WELL: County Simore	1	
NE X NE X Sec. 13 T. 1 M/S R. 7 E/M	-	

Use other side for additional remarks

USGS

IDAHO DEPARTMENT OF WAT 307103 04S-07E-33AAA1 Use Typewriter or Ballpo	REPORT O	6527	Office Us Inspected by Twp Rge	
D0000092			1/41/4	
. DRILLING PERMIT NO.	11. WELL TEST		Lat: : : Loi	ng: : :
Prier IDWH No. 01- 9 1- 14 - 00 2 8- 000	🔅 Pump	🚍 Bailer		ng Artesian
. OWNER;	Yiaid gal. ##m	Drawsiown	12Aubhd Fenes	
ameDave_& Stephanie_Bergh	23			4hrs
ddress_Rt_1 Box 814				
Sity Mtn HomeState_Id_Zip_83647	L			I
	Water Temp		Botto	om hole temp
LOCATION OF WELL by legal description:	Water Quality test o	r comments:		
iketch map location must agree with written location.			Depth first Water	
N	12. LITHOLOGI	C LOG: (Des	scribe repairs or aba	ndonment) <sub>v</sub>
	Bare			1
Twp. 4 North 🔅 or South 🗴	Dia. Prom To	Hemarks: Litho	logy, Water Quality & T	emperature Y
Rge. 7 East X or West		Top Soil		
Sec. 33 1/4 NE 1/4 NE 1/4		Clayey Br		
Gov': Lot County Elimore top acres		Sandy Gra	ivel <u>p</u> =	CEIVE
Lat: : Long: · :		Broken La	Wa Nua	VLIVE
S Address of Well Site OFF Hickney 30 past		Gray Lava		
Claser Ro. cay Mtr. Hame		Brown Cir	iders <b>JU</b>	<u>. 3 1 199</u> 7
"Give at least nome of road + Diviarios to Resour Landmark:	12" 29 34	Gray Lava		
1		Brown Cir	der Departmen	l of Water Holson
	12" 44	Gray Lava	/ Soft-med	
USE:	12" 110	Spoatic/	return	
© Opmestic () Municipal C' Monitor ⊡ Irrigation	12" 110 131	Brown Cir	ders/Clay	
C Thermat			inder	
			<u>een Lava Sofi</u>	. – – – –
	12" 153 160	Drown Cir	ider	«
X New Well □ Modify □ Abandonment □ Other	12" 160 170	Crav Ister	Soft	
i. DRILL METHOD		Browncind		
🕵 Air Rotary 🗇 Cable 🔅 Mud Rolary 🖾 Other		Sand & Gr		
. SEALING PROCEDURES			sand & Gravel	5 01-1
SEAL/FILTER PACK AMOUNT METHOD	6" 235 238	Tan Clay	Sand & GLAVE!	a chay
the second	6" 238 254	Comental	Sand & Gravel	
	6" 254 278	<u>cemencea</u>	Sand & Grave	
Bentonite 0 18 300t Overbore			avel & Clay	
			<u>Clay</u>	<b></b>
	6" 306 310	rine Sang	& Gravel	····· <del>···</del>
Vas drive shoe used? DXY III N Shoe Depth(s)		Cemented	Sanc D	ECEIVEI
Vas drive shoe seal tested? ((Y g/N How?	www.	Clay		
CASING/LINER:			all Gravel	UL 25 199
Diamotor From To Gauge Material Casing Liner Welded Threaded	6" <u>346 360</u>	ran Sandy	Clay J	
8 3/8 4 18 250 Steel x	6" 360 378	<u>_vsid_der</u>	······	WESTERN REGI
65/8 <u>25030250 Steel</u> 32	6" <u>378</u> <u>380</u> 6" <u>380</u> <u>485</u>	<u>ran Clay</u>	1	WESTERN MEGH
	611 40E E00	Sand & Gr	avel atom Altreat	····
ength of Headpipe Length of Tai/pipe	6" 485 509	Biue Sand	stone & Clay	
PERFORATIONS/SCREENS	6" 509 512			X
Perforations Method	6" 512 540		with Sand	
Screens Screen Type	Completed Depth			(Measura
From To Silet Size Number Downster Matsoid Casing Logi	Date: Started 6	/16/97	Completed	7/18/97
			······	
	13. DRILLER'S			
	<ul> <li>I/We certify that all n the filme the rig was</li> </ul>		onstruction standards	were complied
	are nime the ng was	removed.		
	Firm Name Hidd	lieston & Sc	n, Anc	Firm No. 35
0. STATIC WATER LEVEL OR ARTESIAN PRESSURE:	.4/	1 100	7577	Firm No <u>3</u> ate_ <b>_7/22/9</b>

control devices:

and Supervisor or Operator\_ \_\_\_\_\_ Dare\_\_\_\_\_ (Sign once if Firm Official & Operator)

.

FORWARD WHITE COPY TO WATER RESOURCES

FLENNINO	400742 05S-07E-03ADB1			]			
ţ	REPORT OF W State o	f Idah	¢			EIVE 22 1967	D
State law requires that the Engineer within 30 days after	this report shal completion or a	l be f bandon	iled ment	with the of the we	State Recl Department	amation Of Keclama	tion
WELL OWNER: HOWARD K. FL	ermi rv 4	Size depth	of dr of w	vell:	e: 8 2 Stan 441	Total ding wate	r
Address Abalestate	Interfer	- Fahr.		° Test	delivery: Bail		gpa
Owner's Permit No. 6334 NATURE OF WORK (check): Replace	ement well	Size	of pu	imp and mo	tor used t	o make te	
New well Deepened AN Water is to be used for:		Drawd	own:	time of t ft. i surface	est: Artesian p Give f	ressure:	Min <u>.</u> ft. fs
METHOD OF CONSTRUCTION: Rotan Dug Other	ry 🗌 Cable 📉	]or Contr	gpf olled	a. Shutof 1 by: Valv	f pressure e     Cap	Plug	1
(explain) CASING SCHEDULE: Threaded "Diam. fromft. to	Welded Tt.	Yes		No	well leak ATERIAI	*****	asing: WATER
"Diam. from ft. to "Diam. from ft. to "Diam. from ft. to	ft.	FROM		TOPS		YE	S OR 1
Thickness of casing:	Material:	2		GEAY 1		)	<u> </u>
(explain)		I'al	121	GrAN 6	AVA SAND		
PERFORATED? Yes No perforator used:	Type of	187	203	Gray h	AKA		
	by " t. to ft.		284 397 447	Blue C	JAND A4 ANDU (	ROCK.	
perforations fromf	t. toft. t. toft. t. toft.	447	468	Blue C	SAND LAY		400
WAS SCREEN INSTALLED? Yes [ Manufacturer's name	No	468 515		Blue SA	Nd CA		Ment
	l No. ft. to ft ft. to ft					₹	+
CONSTRUCTION: Well gravel pack	ced? Yes Gravel		1	of W			
placed from ft. to ft. provided? Yes No To ft. Material used in sea	Surface seal what depth?		$\mathcal{N}$	n.A	· · · · · · · · · · · · · · · · · · ·		<b> </b>
Did any strata contain unusabl	Le water? Yes		X	Jolex	······································		
No. Type of water: Depth of strataft. 1 strata off:	Nethod of sealin						
	······································	·		······································	·		
Cemented in place? Yes					·····		
Locate well in sec	tion	<b>+</b>					·
	WELL	Work	stari	ted: <u>7-1</u>		<u> </u>	
Sec 3		Well drill	Dril) ed_um	ler's Stat nder my su	ement: Thi pervision of my)kno	and this	repor
		Addre Signe	ss: <u> </u>	15 38 13124	JE aley		
LOCATION OF WELL: County <u>SE &amp; ME %</u> sec. <u>3</u> T. <b>ME</b> %/S	K <u>MCRE</u> R. <u>7</u> E/#	, arcen	25 N(	·• <u></u>	Jarg:	-14-67	
<i>్ చ్</i> ర్జం	e other side for	addit	ional	l remarks			

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#### REPORT OF WELL DRILLER State of Idaho

04S-07E-18AAA1

Department of Reclamation

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State law requires that this report shall be filed with the State Reclamation Engineer within 30 days after completion or abandonment of the well.

WELL OWNER: Grofsema Name <u>Hapvoy</u>	Size of drilled hole: 201 Total depth of well: 685 Standing water level below ground: 305 Temp. Fahr. ° Test delivery: gpm or cfs Pump? Bail
	level below ground: 305 Temp.
Address Mountain Home, Idaho	Fahr. ° Test delivery: gpm
	or cfs Pump? Bail
Owner's Permit No. 759	Size of pump and motor used to make test:
NATURE OF WORK (check): Replacement well New well Deepened Abandoned	
	Length of time of test: Hrs. Min. Drawdown:ft. Artesian pressure: ft.
Water is to be used for:	prawdown: it. Artesian pressure: it.
METHOD_OF CONSTRUCTION: Rotary Cable X	above land surface Give flow cfs or gpm. Shutoff pressure:
Dug Other	Controlled by: Valve Cap Plug
(explain)	No control Does well leak around casing?
CASING SCHEDULE: Threaded Welded	No control Does well leak around casing? Yes No 104296 DEPTH MATERIAL WATER FROM TO YES OF NO
22 "Diam, from o ft, to en ft.	DEPTH MATERIAL WATER
x fix "Diam. from ft. to ft.	FROM TO YES OR NO
<b>SolutionProperty of the Second Secon</b>	FEET FEET
16 "Diam. from 115 ft. to cre ft.	10 322 (Top Soll
Thickness of casing: Material:	32 45 Gray Lava Broken
Steel K concrete wood cher	45 118 Gray Lava Solid
	118 122 Broken Lava loose Rock
(explain)	122 144 Brown Lava
PERFORATED? Yes No Type of	144 154 Gray Lava 154 161 Porous Lava & Cinders
perforator used:	161 194 Gray Lava
	194 212 Broken Leve & Clev
Size of perforations: 1/4 " by 21/2"	194 212 Broken Lava & Clay 212 238 Hard Lava Blue
Size of perforations: 1/4 " by 2 1/2 " <u>http:</u> perforations from <u>file</u> ft. to <u>ft.</u> 2700 perforations from <u>file</u> ft. to <u>505</u> ft. perforations from <u>ft.</u> to <u>ft.</u> WAS SCREEN INSTALLED? Yes	238 245 Red clay 245 305 Hard Gray Lava
2700 perforations from har ft. to to ft.	245 305 Hard Gray Lava
ft. toft.	305 340 Black Lava Some Water Yes
Perforations from ft. to ft.	340 383 Black & Red Lave Yes
Manufacturer's name TypeModel No DiamSlot sizeSet fromft. toft.	226 470 Black Leva & Cinders
Diam. Slot size Set from ft. to ft.	470 495 Black Lava Hard
DiamSlot sizeSet fromit. toft.	505 525 Green Clay
	525 545 Brown Clay
CONSTRUCTION: Well gravel packed? Yes	545 590 White Clay
No size of gravel Gravel placed fromft. toft. Surface seal provided? Yes No To what depth?	590 655 Sandstone
provided? Yes No To what depth?	690 655 Sandstone 655 660 Fine Gravel Yes
ft. Material used in seal:	660 682 Brown Sandstone
	582 685 White Clay
Did any strata contain unusable water? Yes	
No. Type of water: Depth of strataft. Method of sealing strata off:	
Depth of strataft. Method of sealing	
strata off:	
Surface casing used? Yes No.	
Cemented in place? Yes No	
Locate well in section	
LOCATE WELL IN SECTION	
	Work started: Jan. 10 , 1960
	Work finished: Aug. 25, 1966
	Well Driller's Statement: This well was
Sec.	drilled under my supervision and this report
	is true to the best of my knowledge.
	Name: Paul Vollmer & Son
	Address: Aberdeen, Idahe
	Signed by: and John of
L.,	License No. Date:
LOCATION OF WELL: County Rimore	171
NE X NE X Sec. 18 T. 4 XX S R. 7 E/WK	
Use other side for	additional remarks
+ -	

376268 For 04S-07E-17CAB1 PARTMENT OF WATER RESC		CES				Office Use Or ID No. 0054			
WELL DRILLER'S REPORT					Twp	ected by Rge	Sec		
1. WELL TAG NO. D 0039655						_ 1/4 1/4			
Water Right or Injection Well No.	12. \		TESTS:		Lat:	: : Long:	: :		
	L	L) Yield gal	Pump	Bailer Brawdow	A 🗌	r Enowing Art	esian Tim		3
2. OWNER: K. Kon Construction Name K. Kon Construction Address 10440 Hury 95 City Payette State Tot Zip 83/dol		TIBIC YE					<b>, II</b> 1		
Address 10440 Huny 95		-		Lost re	turn	15			
City Payette State Id Zip 83dol				ļ					
3. LOCATION OF WELL by legal description:						Boltom	hole temp	·	
You must provide address or Lot, Bik, Sub or Directions to well.	Wate	r Quali	ty test or	comments:					
Twp North 🗔 ar South 🔀		TUM	Ocic I	001 /0					
Age7 East X or West □ Sec. 17	Bore	1						Wat	
Sec. 17	Dia.	From	Τυ			, Water Quality & Tempo	erature	Y	N
Lat: : : Address of Well Site 2785 Garza Dr	10	0		YOP B	oil				
Address of Well Site 2/85 Gov2a kor	11 H	3	9	clay	- 1-00	a			
City Mt Kome.	H	21	60	Site L	sus.				
Lt Bik Sub. Name	6	60	190	Soft L	aus				
	и		193						
4. USE:	*	1	200			· · · · · · · · · · · · · · · · · · ·			
Somestic Difference Municipal Difference Intrigation			20			wa			_
Thermal Dinjection Dother	11					//////////////////////////////////////			
5. TYPE OF WORK check all that apply (Replacement etc.)	4			Soft 4			····		
Xew Well D Modify D Abandonment D Other		ļ							
6. DRILL METHOD:		-							
🛱 ir Rotary 🗆 Cable 🛛 Mud Rotary 🖓 Other								+	
7. SEALING PROCEDURES									
Seal Material From To Weight / Volume Seal Placement Method									
Bendonite O 60 1250 4 over bore_		ļ							
Was drive shoe used?       XY       □ N       Shoe Depth(s)									$\neg$
8. CASING/LINER:	ļ								
Diameter     From     To     Gaugo     Material     Casing     Liner     Welded     Threaded       6°     42     60     250     Skeel     P     9     9     1					DE				
	·				110	ECEIVED			
				<u> </u>	SI	EP 0 2 2003			
Length of Headpipe Length of Tailpipe		ļ							
Packer Y N Type	-				WAT	ER RESOURCES			
9. PERFORATIONS/SCREENS PACKER TYPE		1							
Perforation Method		1							
From To Siol Size Number Diameter Material Casing Liner						·/·			
	Co	mpleted	! Depth		44	/	(Mea	surab	lie)
	Dai	te: Sta	rleđ	8 de	03		8.21	0:	٤
				RTIFICATIO					-
10. FILTER PACK Filter Material From To Weight / Volume Placement Method			lhat all m was remo		onstructio	on standards were com	iplied with a	at the	i.
		-			10 1	killing		1.	2/
	Com	pany N	ame <u>/ 1</u>		çe k	selling	_ hirm No. (		<i>7</i>
11. STATIC WATER LEVEL OR ARTESIAN PRESSURE:	Princ	ipal Dri	lie (	ty la	ye	Date	8:28	0	٤
338         ft. below ground         Artesian pressureib.           Depth flow encounteredft.         Describe access port or control devices:ib.	and Drille	r or Op	erator 12	Truly &	Zyn	٤ Date	8-28	٠Œ	3
Depth flow encounteredt. Describe access part or control devices:									
	Oper	ator I		Principal Drill	er and Ri	Date Date	<u></u>		

Foreign United and Fig Operator Hequired. Operator I must have signature of Driller/Operator II. FORWARD WHITE COPY TO WATER RESOURCES

400814 045-07E-16BBB1 REPORT OF WELL DRILLER State of Idaho State law requires that this report shall be filed with the State Proclamation Engineer within 30 days after completion or abandonment of the well.

WELL OWNER: Name BEKY Corporation	Size of drilled hole: 20 inches Totallon depth of well: 569 feet Standing water
Addressiountain Home, Idaho	level below ground: <u>314 feet</u> Temp. Fahr. <u>50</u> ° Test <u>delivery: 3200</u> gpm
	orcfs Pump? X Bail
Owner's Permit No. 61-7021	Size of pump and motor used to make test:
NATURE OF WORK (check): Replacement well New well X Deepened Abandoned	14" bowls; 12" column and 350 hp motor
	Length of time of test: <u>6</u> Hrs. Min. Drawdown: <u>22</u> ft. Artesian pressure: ft.
Water is to be used for: Irrigation	above land surfacenons Give flow cfs
METHOD OF CONSTRUCTION: Rotary Cable X	lor gpm. Shutoff pressure: non#
Dug Other (explain)	Controlled by: Valve Cap Plug
CASING SCHEDULE: Threaded Welded	
20 "Diam. from 0 ft. to 12 ft.	DEPTH MATERIAL LOT JO WATER
"Diam. from ft. to ft. "Diam. from ft. to ft. "Diam. from ft. to ft. Thickness of casing: r inch Material:	No control X Does well leak around casing?         Yes       No         DEPTH       MATERIAL         FROM TO       YES OR NO         FEET FEET
"Diam. from ft. to ft.	
Thickness of casing: rinch Material:	4 12 Boulders
Steel X concrete wood cother	
	39         54         Brown Lava           54         96         Red Lava
(explain)	96 124 Red Cinders
PERFORATED? Yes No I Type of	124 138 Brown Lava
perforator used:	138 166 Gray Lava
Size of perforations: " by "	166 187 Red Lava 187 208 Brown Lava
Size of perforations:       " by "         perforations from       ft. to ft.         variable       ft.         perforations from       ft. to ft.         variable       ft.         variable       ft.         variable       ft.         variable       variable         variable       va	208 219 Grav Lava
perforations fromft. toft.	219 247 Open Ground
periorations fromft, toft,	247 278 Gray Lava 278 294 Brown Lava
	29/ 311 Grav Lows
Manufacturer's name	311 B18 Cinders yes
Manufacturer's name TypeModel No DiamSlot sizeSet fromft. toft.	318         332         Red Lava           332         357         Cinders         yes
DiamSlot sizeSet fromft. toft.	332 357 Cinders yes 357 884 Red Lave
CONSTRUCTION: Well gravel packed? Yes	384 390 Gray Lava
No. A size of gravel Gravel	390         402         Cinders         yes           402         405         Gray Lava
No. A size of gravel Gravel placed from ft. to ft. Surface seal provided? Yes X No To what depth?	205 129 Cindema and Class
<u>12</u> ft. Material used in seal: Coment	429 458 Cinders yes
	KANG ANG I (ITAV LAVA (Hend)
Did any strata contain unusable water? Yes	
No. A Type of water: Depth of strataft. Method of sealing	
strata off:	
Surface casing used? Yes X No.	
Cemented in place? Yes X No	
Locate well in section	
I	
· · · · · · · · · · · · · · · · · · ·	Work started: 1-29-68
	Work finished: 4-18-68
Sec	Well Driller's Statement: This well was
	drilled under my supervision and this report is true to the best of my knowledge.
	Name: B.B. Gailey
┍╴┈╶┾┈┈┈╺┼╺╴╺╸╺┥┈╶╸╺┥	Address: 905 N 10th E, Mtn Home, Idaho
	Signed by: AR Galou
	License No. 89 Date: 5-14-68
LOCATION OF WELL: County Elmore	
<u>NW X NW X Sec. 16 T./S</u> VS R. 7 E/S	

Use other side for additional remarks

USGS

- H	ZI_OY	7 <b>80</b>	No	County_ ELMORE	a Januarian	<b></b>	
Owner	ARVEY OF	ROEFSEM	A	······································	Locate -	well in section	I 
Address_S	E. MT.	HOME,	IDAHO			• •	
Driller H	ERRY L.	JOHNSO:	N DRILLI	NG & PUMP CO.	NW4	NE14	
Address 2	211 NO.	26th,	BOISE,	IDAHO P.O.BOX 5042			
Well locat	ION SW V	<u>s 2-</u> %	see the	., <u>T. 4</u>			
Size of dril	led hole 20	<u>) H</u>			5W1/4	- <b>5</b> £14	
<u></u>		~		Total depth of well 862 Tt	1,		
Give depth	to standing	water from	m the ground	350Water tempPehr.	-		
On "Pumpi	ing Test' de	alivery was	<u>1350 g.p.n</u>	n. or		T .	: "
	np and mate		•	2" Bowls	<u> </u>	-	<u> </u>
Longth of H	ime of test						
If flowing	weil, give f	lew	_c.f.s. or	g.p.m. and of shut off processre			
if flowing v	vell, describe	ed control v	works		· ·		
Water will	be used for.	TREEL	ATTON.	TYPE AND BIZE OF VALVE,		8	
			_Casing mat	erial		-	•
	ength and lo		-	(STEL. CONCRETE, W	<u>T0</u>	•	
		2		(CASING 12" IN DIAMETER ON LESS, GIVE CASING OVEN 12" IN DIAMETER, GIVE DI	INSIDE DIAMET	ZH: R)	
		<u> </u>					a and a second s
<u> </u>	,			CASING RECORD	•		
Diam. Casing 12 <sup>st</sup>	From Foot Burface	To Feet 630	Length 6301	Remarks—seals, gr	outing, etc.	·······	
<b>∠∠</b> " ¦	665	775	1001			<u>`</u>	
· 8# )				······································			
			~				
· 8# ]						······································	
· 8# ]	· · ·						and s

WELL LOG

From Foet	To Féat	Type of Materia)	Water-bearing Yormation Ana. Yes or No	Casturg Perforaturg Aus. Yes or No
<b>0</b> 40 42 50 60	40	hard lave		····-
40	42	open dry crevice	Į	
42	50 60	hard lava soft rock	·	
-50	81	hard lava, tools run off, bit battered. crevice	an 81	
00	U UL	trickle of water at 82		-
81	90	softer rock		
90	96	red cinders		
96	103	rock 7 brown clay, firm		
.io3	125	hattd lave		
125	129	cinders cavey		
129	133 141	hard basilt		<u> </u>
133	141 114	brocken rock & cinders cavey crevice bad bole		
		hard lava, shot hole at 155		
176	176 178	broken rock		
90 96 103 125 129 133 141 144 176 178	202	medium lava		
-202	206	extra hard lave	ļ	
206	208	crevice, lost water		
	211	broken rock & crevice		
211	220	medium basalt, holding water		
-220	222	soft streak, lost vater	<u>}</u>	
222	227 -	soft & hard streaks, wont hold water hard & soft streaks, holding water again		
-227	234	red cinders		
232	255	hard lava, very rough going		<u> </u>
255	262	cinders & broken rock		
-262-	280	broken rock lost water & cuttings	ļ	
222 232 232 255 262 280	297 303 307	ftrm leve		
-271	<del> </del>	picked up drillings pomous rock, water at 387		
303	307	soft material, lost water		
- <u>307</u> 325 -335	325 335 348	hole very bad, shooting every 6 Ft.	1	
325	332	medium lava very hard lava, bit batters		
-332	-0-LC-	If more space is required use Sheet No. 2		

#### WELL DRILLER'S STATEMENT

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This well was drilled under my supervision and the above information is true and correct to the best of my know-

ledge and ballef.

N

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<u>ung</u> man Signed āy.

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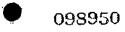
25 License No.

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#### SHEET NO. 2

Well OwnerH. Groefsena

Well Driller\_JOHNSON\_DRILLING\_

Well Location \_ ELMORE COUNTY

From Feet	To Feat	Type of Material	Wates-bearing Formation Ans. Yes of No	Casiby Casiby Furforated Ann. Tra or Na
34	351	medium lawa broken rock, water yellow clay hard lava with soft streaks cinders yerw hard grey lava		
351	353	broken rock, water		
353		yollow clay	·	
305	401	hard lava with soft streaks		<b>.</b>
101 101	405 445	Cluders		
445	448	vern hard grey lava black lava some water coming in		
<u>ұ</u> ц8	455	i red rock		
455	4.75	broken rock, water bearing d well Sept 29-61 set bowls at 430' 800GPM		
<u>+75</u>	teste	d well Sept 29-61 set bowls at 430' 800GFM		
472	482 508	LOST cuttings more water coming in		
402 508	1 200	hard & soft layers very few cutting more water broken rock water bearing		
512	512 558	firm black lava		
3361 33601 555 555 558 558 558 558	588	blue shale		
588	590	grev soft clev		
590	628	soft grey sticky clay caving		
628 41.5	6 <u>45</u> 687	very hard		
687	691	nedium firm clay per & larger gravel		
691	752	hard sand stone with 1 to 2' thick layers of cla	37	
752	752 765	brown sticky clay cavey	3	
765	772	brown sticky clay cavey grey clay and chelk cavey		
772	775	unable to hold open, set 100 Ft. 8" liner shoe ( layers of sand stone and clay	n os	h-ond
590 628 645 687 691 752 765 772 775 835	772 775 835 862	sticky clay bottomed hole		
	دېږې د مېسور در د		u	n
	to	475 Ft. 20" bole		
	1	475 to 670 169 bole		
<b>.</b>		630 to 775 12 " hole 775 to 862 8" _ hole(bottom)		
		775 to 862 8" _ hole(bottom)		
	L			
	<b> </b>			
	<b></b>			
	<b> </b>			
		usris		
		······································		
	<u> </u>	SWSES9457E		

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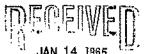
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Wall Log Form

424833 04S-08E-14AAA1

j.

090	394	2



## WELL LOG AND REPORT OF THE JAN 14 1865 MET

	Bailars						Locate wel	in section
Address	10 July 10 Jul	otee						
Driller							NW 14	NE 1/4
Address	Box 489	Caldwe	11 Idah	D				
Well loca	tion DE	WE .	1/4 Sec. 12/4	Ŀ, <b>⊤<u>5</u>88</b>		e/y	SW1/4	SE 1/4
	<u>115 13</u>	in.			pth of well_58	3		
Give dept	h to standin	ig water fri	om the grou None	nd 416 - y	Vater temp. <u>48</u>	ºFahr.		
					c.f.s. Drawdown			
lize of pu	mp and mol	- <u>nr</u> .,			. Eovla			·····
ength of	time of test.	و	hours		ninutes	·		
f flowing	well, give	flow	c.f.s. or	g.p.m. q	nd of shut off p	ressure		
f flowing	weil, descrii	hed control	works	<u></u>	(TYPE AND SIZ			
Vator will	ba used fo	r			Weight of casing	E OF VALVE, I	421b.	
					-weißen et restui	8 fen. meat	1001	
		200					'	
					steel			
					steel			
)iameter, l		ocation of a	casin <u>y 3</u>	43 ft. 16	Steel (STEEL, C 10, 26 2" IN DIAMETER OF OVER 12" IN DIAMET	ONCRETE, WO	DOD. ETC.) 250 Line: INSIDE DIAMETERI TSIDE DIAMETERI	
liameter, l	ength and l	ocation of a	casin <u>y 3</u>	43 ft. 16 (CABING 1) (CABING 1) (CABING 1)	steel	ONCRETE, WO	DOD. ETC.) 250 Line: INSIDE DIAMETERI TSIDE DIAMETERI	
)iameter, l	ength and l	ocation of a	casin <u>y 3</u>	43 ft. 16 (CABING 1 0493	steel steel, c in, 26 in, 26 in Diameter of oven 12" in Diame 14 in lip	ONCRETE, WO	DOD. ETC.) 250 Line: INSIDE DIAMETERI TSIDE DIAMETERI	
)iameter, l	ength and l	ocation of a	casin <u>y 3</u>	43 ft. 16 (CABING 1 0493	steel (STEEL, C 1n, 26 2" IN DIAMETER OF OVER 12" IN DIAME 14. in 11p RECORD	ONCRETE, WO	DOD. ETC.)	
Diamster, I G Diam.	From	ocation of t	465 te	43 ft. 16 (CABING 1 0493	steel (STEEL, C 1n, 26 2" IN DIAMETER OF OVER 12" IN DIAME 14. in 11p RECORD	ONCRETE, WO	DOD. ETC.)	
Diamster, 1 Diam. Casing	From Foot	To Foet	465 te	43 ft. 16 (CABING 1 0493	steel (STEEL, C 1n, 26 2" IN DIAMETER OF OVER 12" IN DIAME IA in lip RECORD Remarks	ONCRETE, WO 11. II LESS, GIVE TER, GIVE OU IOT 	UDD. ETC.)	
Diam. Casing 16	From 0	To Foct	465 te	43 ft. 16 (CABING (CABING )	steel (STEEL, C 1n, 26 2" IN DIAMETER OF OVER 12" IN DIAME IA in lip RECORD Remarks	ONCRETE, WO 11. II LESS, GIVE TER, GIVE OU IOT 	DOD. ETC.)	
Diameter, 1 Q Diam. Casing 16	From 0	To Foct	465 te	43 ft. 16 (CABING (CABING )	steel (STEEL, C 1n, 26 2" IN DIAMETER OF OVER 12" IN DIAME IA in lip RECORD Remarks	ONCRETE, WO 11. II LESS, GIVE TER, GIVE OU IOT 	UDD. ETC.)	
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### IDAHO DEPARTMENT OF WATER RESOURCES WELL DRILLER'S REPORT

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\* Signature of Principal Driller and rig operator are required.

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### IDAHO DEPARTMENT OF WATER RESOURCES WELL DRILLER'S REPORT

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# **Appendix B**

Historical Groundwater Flow Gradient Maps

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OF THE MOUNTAIN HOME AREA, ELMORE COUNTY, IDAHO

> Water Information Bulletin No.4 Iaaho Department of Reclamation

> > July 1968

### WATER INFORMATION BULLETIN NO. 4

### GROUND-WATER RESOURCE OF THE MOUNTAIN HOME AREA, ELMORE COUNTY, IDAHO

by

Dale R. Ralston

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Hydrologist

and

Sherl L. Chapmon

Geologist

Prepared and Published by Idaho Department of Reclamation R. Keith Higginson State Reclamation Engineer

JULY 1968

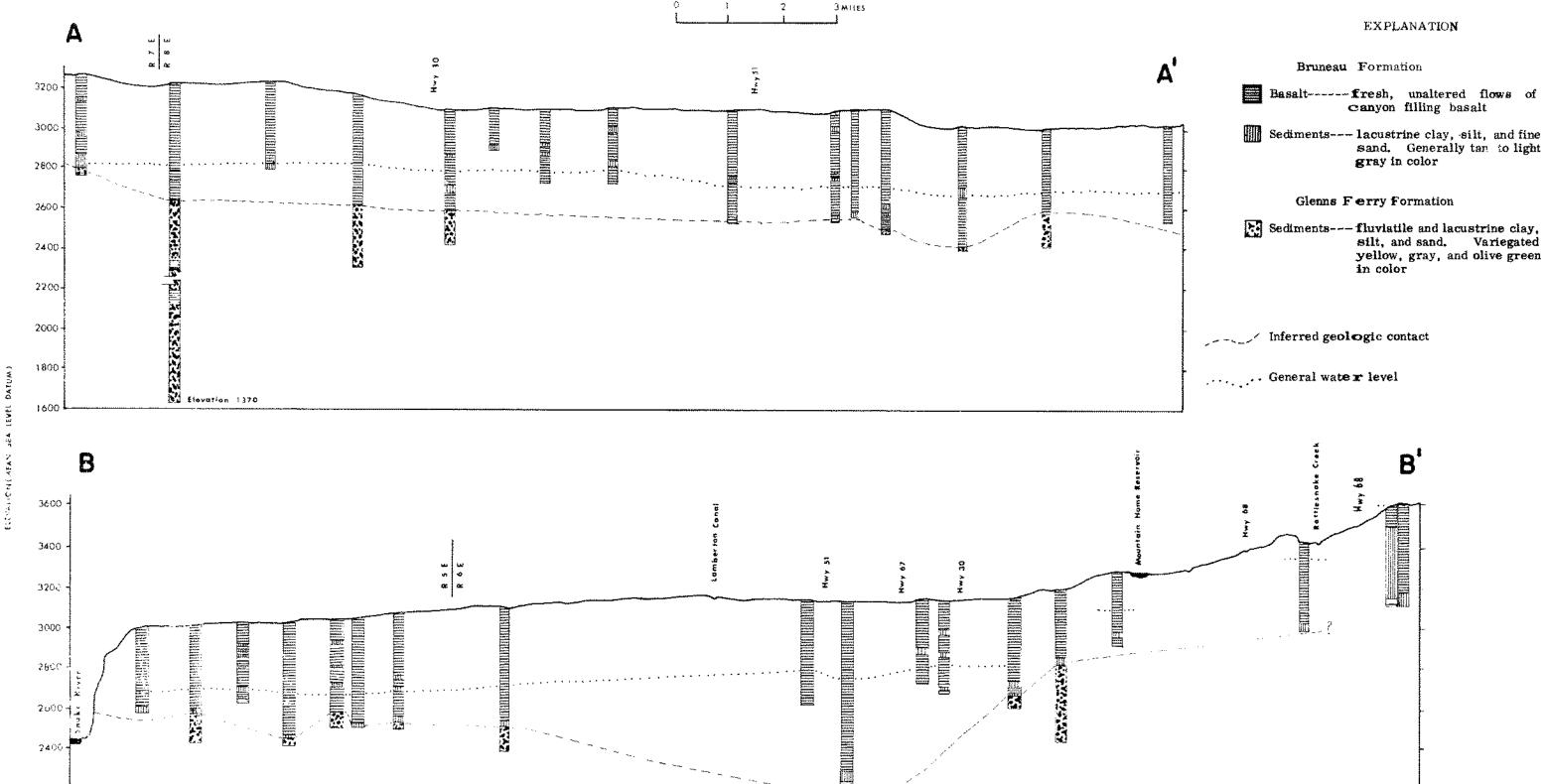
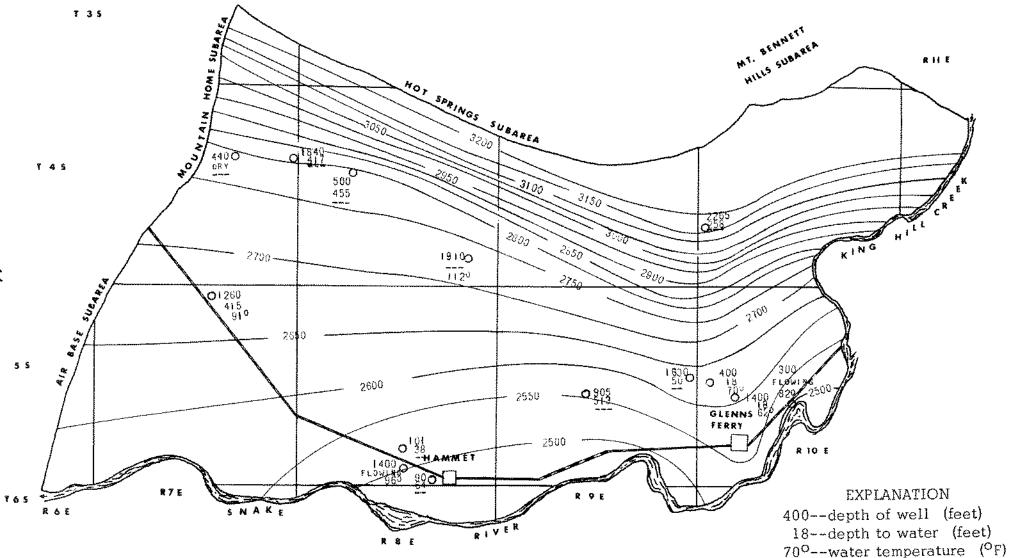
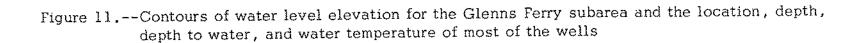


Figure 4. -- Generalized Geologic Cross Sections

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GROUND-WATER QUALITY IN THE WESTERN SNAKE RIVER BASIN, SWAN FALLS TO GLENNS FERRY, IDAHO By D. J. Parliman

U.S. GEOLOGICAL SURVEY

Water-Resources Investigations Report 83-4062

Prepared in cooperation with the IDAHO DEPARTMENT OF WATER RESOURCES

> Boise, Idaho October 1983



UNITED STATES DEPARTMENT OF THE INTERIOR

James G. Watt, Secretary

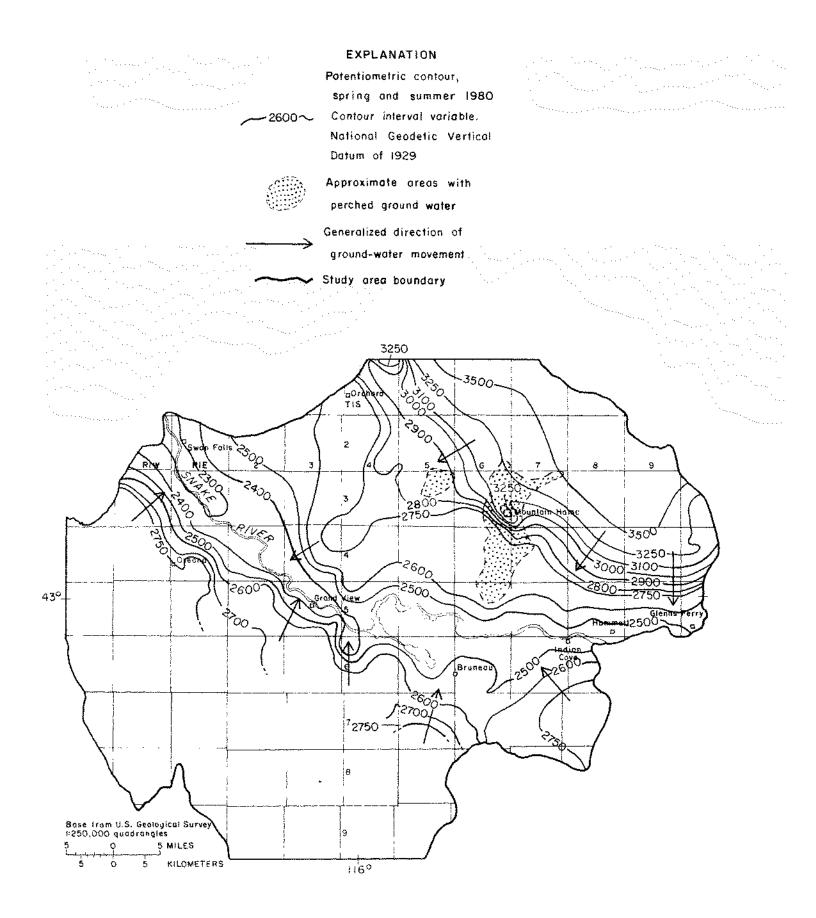
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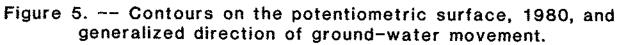
Dallas L. Peck, Director

For additional information, write to:

Acting State Office Chief U.S. Geological Survey, WRD 230 Collins Road Boise, ID 83702 (208) 334-1750 Copies of this report can be purchased from:

Open-File Services Section Western Distribution Branch U.S. Geological Survey Box 25425, Federal Center Denver, CO 80225 (303) 234-5888





# **Appendix C**

Groundwater Sampling and Analysis Plan

### 2024 Bennett Road Landfill Groundwater Sampling and Analysis Plan

Prepared for Elmore County



July 2024

## Parametrix

### 2024 Bennett Road Landfill Groundwater Sampling and Analysis Plan

Prepared for

Elmore County 150 S 4th E Street Mountain Home, ID 83647

Prepared by

**Parametrix** 7761 W Riverside Drive, Suite 201 Boise, ID 83714-5044 T. 208.898.0012 F. 1.855.542.6353 www.parametrix.com

July 2024 | 553-7443-006

### Citation

Parametrix. 2024. 2024 Bennett Road Landfill Groundwater Sampling and Analysis Plan. Prepared for Elmore County by Parametrix, Boise, Idaho. July 2024.

### Certification

The technical material and data contained in this document were prepared under the supervision and direction of the undersigned, whose seal, as a professional engineer licensed to practice as such, is affixed below.

Prepared by Shira DeGrood, PG	
Checked by Michael Brady, LG, LHG	
Approved by Tiffany Neier, PE	

July 2024 | 553-7443-006

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#### APPENDICES

- A Standard Operating Procedures and Checklists
- B Bennett Road Landfill Quality Assurance Project Plan

# **Acronyms and Abbreviations**

ASTM	American Society of Testing and Materials
BRL	Bennett Road Landfill
CFR	Code of Federal Regulations
EPA	Environmental Protection Agency
IDAPA	Idaho Administrative Code
IDEQ	Idaho Department of Environmental Quality
MCLs	maximum contaminant levels
QAPP	Quality Assurance Project Plan
SAP	Sampling and Analysis Plan
SOP	standard operating procedure
Subtitle D	Federal Regulation 40, CFR Part 258, Solid Waste Disposal Facility Criteria
SWFA	Idaho Solid Waste Facilities Act
VOC	volatile organic compound

# **1.** Introduction

This groundwater sampling and analysis plan (SAP) describes the proposed groundwater monitoring program for the Bennett Road Landfill (BRL) in Elmore County, Idaho. The facility is located approximately 6 miles southeast of Mountain Home along I-84. The address of the landfill is 6100 SE County Landfill Road, Mountain Home, Idaho, 83647.

The groundwater monitoring program is designed to meet the applicable federal and state regulations and additional criteria established by the Idaho Department of Environmental Quality (IDEQ). The groundwater monitoring program meets federal regulations for municipal waste landfills (Federal Regulation 40, Code of Federal Regulations [CFR] Part 258, Solid Waste Disposal Facility Criteria [Subtitle D] and the Idaho Solid Waste Facilities Act [SWFA; §39-7410]). This groundwater monitoring program does not include procedures for leachate and underdrain monitoring or for surface water monitoring.

## 2. Groundwater Monitoring Locations

Two point of compliance wells are proposed for installation downgradient of current Cell 1 and future Cell 2 of the BRL. The proposed monitoring wells will be installed into the uppermost aquifer below the facility and appropriately placed to detect potential impacts from leachate that may enter the underlying groundwater system.

Cell 2 will be constructed with a leachate collection system and the leachate may be monitored once Cell 2 construction is complete. Analytical data on leachate may be assessed to determine likely indicator parameters for future groundwater statistical assessment.

Well 04S-07E-14AAA1 is the Facility Supply Well and is located upgradient of Cells 1 and 2. It is installed into the regional shallow aquifer below the facility and appropriately placed to provide background information on groundwater conditions prior to migration under Cells 1 and 2. The sample collection location will be a faucet that is connected to the system; the target sampling faucet will be determined prior to the initial sampling event. Additional wells such as House Well 04-07E-13AAD3 may also be present upgradient, northeast of the facility on the County-owned parcel and completed in the regional shallow aquifer. If this well is intact, it may also be added to the monitoring well network for background water quality monitoring.

## **3.** Sampling and Analysis

This chapter describes the monitoring schedule and procedures for water level measurements, sample collection, laboratory test parameters, and quality assurance.

## 3.1 Monitoring Schedule

Groundwater monitoring will be conducted quarterly for the first eight sampling events to determine background conditions for statistical analysis. Once background conditions have been established, future monitoring events will be compared to the background conditions for the statistical evaluation, either interwell or intrawell comparisons, following the U.S. Environmental Protection Agency's (EPA's) Unified Guidance (2009) and IDEQ's Statistical Guidance for Determining Background Ground Water Quality and Degradation (2014).

#### **3.2** Water Level Measurements

Static groundwater levels will be measured during each sampling event in the monitoring wells. Water levels will also be measured in the Facility Supply Well at the time of sampling; however, this may not be a true static water level due to the utilization by the facility. If any House well (Wells 04-07E-13AAD1 through -13AAD4) is present, static water levels will also be measured.

Depth to groundwater will be measured in accordance with the standard operating procedure (SOP) for Static Water Level Measurement, presented in Appendix A. Water levels will be measured to the nearest 0.01 foot using an electronic water level indicator. Water levels will be measured before, during, and after purging to assess drawdown effects at each well.

#### 3.3 Sample Collection Procedures

Dedicated submersible groundwater sampling pumps will be installed in MW-1 and MW-2. The wells will be purged using a low-flow purging technique in accordance with the SOP for Groundwater Sampling presented in Appendix A. Groundwater stabilization parameters will include temperature, pH, specific conductivity, visual color, and visual turbidity.

The Facility Supply Well has a dedicated submersible groundwater pump. Water from this well will be accessed through a faucet. The faucet will be turned on to a flow rate of approximately 500 milliliters per minute. The well will be purged and sampled following stabilization techniques in accordance with the SOP for Groundwater Sampling presented in Appendix A.

If a House well is present, a SOP will be established to allow sampling of the well.

Samples will be collected when field parameters stabilize in accordance with the SOP for Groundwater Sampling presented in Appendix A. Samples to be tested for dissolved metals will be field-filtered through 0.45-micron disposable filters.

#### **3.4** Test Parameters

Samples will be tested in the field for the following parameters:

- Temperature
- pH
- Specific conductivity
- Dissolved oxygen
- Oxidation-Reduction Potential
- Turbidity

All laboratory analyses will be performed by an EPA-certified laboratory that will provide sample bottles with the appropriate preservatives. Analyses will be performed in accordance with standard EPA analysis methods (EPA Publication Number SW-846, Test Methods for Evaluating Solid Waste - Physical/Chemical Methods [EPA 1996]).

Detection monitoring parameters for groundwater will include those summarized in Table 1. Parameters for groundwater analysis will include those metals and volatile organic compounds specified in Appendix I of 40 CFR part 258 and additional cations/anions that may be indicators of leachate.

Required sample containers, preparation, preservatives, and holding times for the test methods will be as specified in the RCRA Technical Enforcement Guidance Document (EPA 1986), and Standard Methods for the Examination of Waste and Wastewater (APHA et al. 1989) and are specified in the quality assurance project plan (QAPP, presented in Appendix B).

	Parameters Required by Subtitle D (App	pendix I)
Metal Constituents <sup>1,2</sup>		
Antimony	Arsenic	Barium
Beryllium	Cadmium	Chromium
Cobalt	Copper	Lead
Nickel	Selenium	Silver
Thallium	Vanadium	Zinc
Organic Constituents <sup>3</sup>		
Acetone	Acrylonitrile	Benzene
Bromochloromethane	Bromodichloromethane	Bromoform
Carbon disulfide	Carbon tetrachloride	Chlorobenzene
Chloroethane	Chloroform	Dibromochloromethane
1,2-Dibromo-3-chloropropane	1,2-Dibromoethane	1,2-Dichlorobenzene
1,4-Dichlorobenzene	Trans-1,4-Dichloro-2-butene	1,1-Dichloroethane
1,2-Dichloroethane	1,1-Dichloroethene	Cis-1,2-Dichloroethene
Trans-1,2-Dichloroethylene	1,2-Dichloropropane	Cis-1,3-Dichloropropene
Trans-1,3-Dichloropropene	Ethylbenzene	2-Hexanone
Bromomethane	Methyl chloride	Dibromomethane
Dichloromethane	2-Butanone	Methyl iodide
4-Methyl-2-pentanone	Styrene	1,1,1,2-Tetrachloroethane
1,1,2,2-Tetrachloroethane	Tetrachloroethene	Toluene
1,1,1-Trichloroethane	1,1,2-Trichloroethane	Trichloroethene
Trichlorofluoromethane	1,2,3-Trichloropropane	Vinyl acetate
Vinyl chloride	Xylenes	
	Additional Inorganic Indicator Parame	eters <sup>2,4</sup>
Chloride	Nitrate	Calcium
Sulfate	Magnesium	Sodium
Potassium	Bicarbonate alkalinity	

Table 1. Detection Monitoring Parameters for Groundwater Samples	
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Samples will not be field-filtered prior to laboratory analysis.

Metals will be tested by U.S. Environmental Protection Agency (EPA) Method 6010 or 7000 series or equivalent.

Organic constituents will be tested by EPA Method 8260 or equivalent.

Additional constituents will be tested by Standard Method 2320 and EPA Method 9056A or equivalent.

## 4. Data Analysis and Reporting

#### 4.1 Data Evaluation

#### 4.1.1 **Quality Assurance/Quality Control Evaluation**

Procedures for quality assurance/quality control evaluation of the data are presented in the QAPP (Appendix B).

#### 4.1.2 Comparison to Groundwater Quality Criteria

The groundwater data will be compared to applicable state and federal groundwater quality criteria. Applicable criteria are federal maximum contaminant levels (MCLs), Idaho Regulations for Public Drinking Water Supplies (Idaho Administrative Code [IDAPA] 58.01.08), and Idaho Groundwater Quality Standards (IDAPA 58.01.11).

#### 4.1.3 Statistical Evaluation of Data for Background Characterization

The statistical evaluation program will be developed in accordance with Subtitle D (Section 258.53[g]) and using guidance provided in Statistical Guidance for Determining Background Ground Water Quality and Degradation (IDEQ 2014); American Society of Testing and Materials (ASTM) standard D6312-17 Standard Guide for Developing Appropriate Statistical Approaches for Groundwater Detection Monitoring Programs at Waste Disposal Facilities (ASTM 2017); and Statistical Analysis of Groundwater Monitoring Data at RCRA Facilities (EPA 2009).

After eight quarters of data is collected from MW-1, MW-2, and the Facility Supply Well, and other wells if present. Data will be assessed following the above guidance documents. The assessment will include an analysis of interwell versus intrawell comparisons. Recommendations for the long-term monitoring program and associated statistical assessment will be made at that time.

#### 4.1.4 Required Actions Following Findings of Statistically Significant Increases in Constituent Levels

Following background characterization, groundwater will be monitored for potential leachate impacts. In the event that a statistically significant increase over previous data is detected for one or more constituents, additional actions are required by the owner or operator under Subtitle D (Section 258.54(c)). Within 14 days of this finding, a notice must be placed in the operating record. If it cannot be demonstrated that a source other than the landfill caused the contamination or that the statistically significant increase results from error in sampling, analysis, statistical evaluation, or natural variation in groundwater quality; an assessment monitoring program as described in Subtitle D (Section 258.55) must be initiated.

#### 4.2 Reporting

Data reports will be transmitted to the Central District Health Department and IDEQ quarterly. Data reports will consist of a table summarizing the data for that monitoring event and a discussion of the data results. The report will include the field forms, laboratory data package, chain-of-custody forms, data tables, trilinear diagrams, a potentiometric surface map, and a data validation report. Since the

wells are new, any detection of volatile organic compounds (VOCs) in the quarterly monitoring shall be considered a potential release and resampling will be performed to confirm the data. An annual report will be completed each year which will include the quarterly monitoring parameters as well as time series plots, upcoming changes at the BRL, and recommendations for adjustments to the monitoring plan.

Following the eighth quarterly background characterization sampling event, an updated SAP will be submitted that describes changes to the groundwater monitoring program and the statistical evaluation methodology.

## 5. References

- APHA (American Public Health Association), American Water Works Association, and Water Pollution Control Federation. 1989. Standard Methods for the Examination of Waste and Wastewater. 17th edition.
- ASTM (American Society of Testing and Materials). 2017. ASTM D6312-17. Provisional Standard Guide for Developing Appropriate Statistical Approaches for Groundwater Detection Monitoring Programs.
- EPA (U.S. Environmental Protection Agency) 1986. RCRA Ground-Water Monitoring Technical Enforcement Guidance Document, OSWER-9950.1.
- EPA. 1996. Test Methods for Evaluating Solid Waste Chemical/Physical Methods (SW-846). Third edition.
- EPA. 2009. Statistical Analysis of Groundwater Monitoring Data at RCRA Facilities Unified Guidance, EPA 530/R-09-007. March 2009.
- IDEQ (Idaho Department of Environmental Quality). 2014. Statistical Guidance for Determining Background Ground Water Quality and Degradation. March 2014.

# **Appendix A**

Standard Operating Procedures and Checklists

#### **APPENDIX A**

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## Standard Operating Procedure Static Water Level Measurement

## Objective

The objective of this standard operating procedure is to describe a method for collecting a static water level measurement. Measurements will be made from groundwater monitoring wells accurate to the nearest 0.01 foot from a standard reference point on the well casing.

### Materials

The following materials are required for the collection of static water level measurements:

- Well keys.
- Electronic water-level indicator.
- Weighted steel tape.
- Paper towels.
- Deionized water.
- Health and safety equipment.

### Procedure

The following steps will be taken during the collection of a static water level measurements:

- 1. Unlock and open well. Verify well integrity.
- 2. Lower electronic water level indicator to the water surface.
- 3. When the sounder indicates that the indicator probe has contacted water, raise and lower the probe to verify exact point at which measurement should be taken.
- 4. Measure the depth to water, to the nearest 0.01 foot, from the reference point (notch or mark on well casing).
- 5. Record the measurement, to the nearest 0.01 foot, in the field notebook or on the Sampling Field Data Sheet.
- 6. Measure total well depth to the nearest 0.1 foot using water level indicator.
- 7. Replace well cap and close and lock protective well casing.

#### Decontamination

The following steps will be taken during decontamination of down-hole measuring equipment:

- 1. While winding the equipment up from the well, thoroughly rinse with deionized water.
- 2. Remove excess water from the equipment with clean paper towels prior to rewinding equipment on the reel.

#### Notes

Measurements will be made under appropriate health and safety procedures.

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## Standard Operating Procedure Groundwater Sampling

### Objective

The objective of this standard operating procedure is to describe methods for the collection of groundwater samples from monitoring wells. Groundwater sample collection procedures include equipment cleaning, water elevation measurements, well purging, and sampling.

#### Materials

The following materials will be used for collection of groundwater samples:

- Sample containers.
- Specific conductivity, pH, and temperature meter and probes.
- Electronic water-level indicator.
- Pump controller.
- Filters for dissolved metals, as needed.

Prior to the sampling event, all equipment which will be placed in the well or come in contact with the groundwater sample will be disassembled and cleaned using the procedure described in the Standard Operating Procedure Decontamination section.

#### Procedures

- 1. Prior to purging a monitoring well, measure the depth to water using the method described in the Standard Operating Procedure Static Water Level Measurement section.
- 2. Unlock well and remove cover.
- 3. Measure initial water level from reference point to the nearest 0.01 foot. Reconfirm measurement and record on field sampling data sheet.
- 4. Start pumping well at 200 to 300 milliliters per minute (ml/min) using dedicated pump. Record on field sampling data sheet.
- 5. Monitor indicator parameters (pH, specific conductivity, and temperature) every 3 to 5 minutes during purging of well and record on field sampling data sheet. Additional indicator parameters may include oxidation-reduction potential, dissolved oxygen, and turbidity. The well is considered stabilized and ready for sample collection when the indicator parameters have stabilized within 10% of the previous reading for three consecutive readings. In general, the order of stabilization is pH, temperature, and specific conductance, followed by oxidation-reduction potential, dissolved oxygen and turbidity.
- 6. Collect samples using dedicated discharge hose directly into prelabeled sample containers at a flow rate between 200 and 300 ml/min. All sample containers should be filled with minimal turbulence by allowing the groundwater to flow from the tubing gently down the inside of the container. Volatile organic compound (VOC) samples should be collected so that no headspace exists to prevent air from remaining in the bottle; after capping, invert and tap the container to ensure no air bubbles are present. Field-filter samples to be tested for dissolved metals through a 0.45-micron membrane filter immediately before filling sample containers.

- 7. Close and lock well.
- 8. Dispose of purge water on the ground surface.
- 9. After purging is complete, begin sample collection. When a pump is used for sampling, operate with as little fluctuation in pumping rates as possible to minimize turbulence and aeration of the pump effluent. Samples will be collected in the following order to minimize volatilization:
  - a. Volatiles.
  - b. Inorganics.
  - c. Metals.
- 10. Fill sample containers for volatile samples so that no headspace exists. After capping, invert the container and tap to verify that no air bubbles are present. For parameters other than volatile organic compounds, fill the sample container to within 2–5 cm of the top, in a manner to minimize aeration. When sampling for dissolved metals, filter the sample using a peristaltic pump and in-line disposable 0.45-micron filter.
- 11. Label all sample containers at the time of sampling. Sample labels will include the following information:
  - a. Project name and number.
  - b. Sample station.
  - c. Sample number.
  - d. Date and time of sample collection.
  - e. Sampler's initials.
  - f. Analyses requested.
- 12. Store all samples at approximately 4°C for transport to the laboratory under chain-of-custody procedures. Samples that will be analyzed for volatile organic compounds will not be placed in direct contact with ice. Upon completion of sampling, the well will be locked and secured.

#### Decontamination

During field sampling, all equipment surfaces placed in well or in contact with groundwater samples will be cleaned before purging and sampling the next well. The equipment will be cleaned using the method described in the Standard Operating Procedure Decontamination section.

## Standard Operating Procedure Decontamination

### Objective

The objective of this standard operating procedure is to describe decontamination procedures to be followed during the performance of field activities.

#### Materials

The following materials are required for performance of equipment decontamination:

- Scrub brush.
- Alconox® or equivalent soap.
- Deionized water.
- Water tubs.
- Health and safety equipment.

### Procedures

The following steps will be taken during decontamination of equipment and materials which may affect sample quality:

- 1. Scrub with non-phosphate detergent.
- 2. Rinse thoroughly with deionized water.

#### Notes

Decontamination wastes will be disposed of according to project-specific considerations.

Decontamination will be performed under Level D health and safety procedures. Site-specific conditions may require additional health and safety precautions.

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## Groundwater Monitoring Procedures Checklist

### Two Weeks Prior To Sampling Event

- 1. Check equipment and sampling kits 2 weeks before scheduled sampling:
  - a. Does the kit contain everything on the checklist? If not, order or purchase.
  - b. Are pH buffers or conductivity standard past their expiration date? If so, order new solution.
  - c. Is pH fill solution within 1 inch of fill hole? If not, add KCI.
  - d. Go through calibration (Step 3) and correct any problems if a probe will not calibrate properly.
  - e. Make sure all other equipment is operating properly.
- 2. Order sample containers from laboratory. See Quality Assurance Project Plan (QAPP) for Sample Container Request Forms.

## One Week Prior To Sampling Event

- 1. Print sample labels and field data sheets.
- 2. Check samples containers, affix labels, separate bottles into groups for each location.

## Day Of Sampling

- 1. Calibrate pH/Conductivity Meter (instructions provided for Corning Checkmate):
  - a. Rinse pH probe with deionized water, insert into pH 7 solution, and press "cal."
  - b. When readout goes to 7.00, rinse again, insert into pH 4 solution, and press "cal" again.
  - c. When readout goes to 4.00, rinse tip thoroughly, replace rubber cap filled with pH 7 buffer, and slide rubber sleeve to cover vent hole.
  - d. Attach conductivity probe to meter and press "cal."
  - e. When readout goes to zero, insert probe into 1413 µmhos/cm conductivity standard, making sure the silver bands are covered without air bubbles in the sleeve, and press "cal" again.
     Note: don't place probe directly into large supply bottle because you may contaminate it. Just pour what you need into a smaller container and close the large bottle immediately.
  - f. When readout stabilizes at 1413  $\mu mhos/cm,$  remove sleeve and rinse probe and sleeve with deionized water.
  - g. Press "mode" twice to turn off meter.
- 2. Measure static water level. See Standard Operating Procedure Static Water Level Monitoring section.
- 3. Purge well. See Standard Operating Procedure Groundwater Sampling section.
- 4. Collect groundwater samples. See Standard Operating Procedure Groundwater Sampling section.

- 5. Collect duplicate sample (to be tested for all parameters):
  - a. Select one groundwater location where duplicate sample will be collected.
  - b. Prepare an extra set of sample containers for all parameters and label them with the sample number and suffix-D (e.g., M5-D).
  - c. Align sample bottles and duplicate bottles by parameter.
  - d. Collect sample for volatile organic compounds (VOCs) then collect duplicate for VOCs. Continue in this manner until all sample bottles are filled.
  - e. Field filter the duplicate sample for dissolved metals using the same tubing and filter as the corresponding sample (see No. 6 below for field filtration procedures).
- 6. Field filter samples for dissolved metals analysis:

#### **Dedicated Pump**

- a. Place 0.45 micron disposable cartridge filter in-line with dedicated discharge hose.
- b. Collect sample directly into the labeled sample container.
- 7. Complete chain-of-custody form:
  - a. Indicate sample locations, date and time of sampling, and required analyses.
  - b. Indicate by bottle and analytical group whether samples were preserved or field-filtered.
  - c. Sign and date.
- 8. Prepare samples for shipment to laboratory in a cooler:
  - a. Tape drain plug of cooler shut on inside and outside.
  - b. If samples are to be shipped, place each sample bottle in a separate sealed plastic bag and place back in the cooler on ice.
  - c. Seal chain-of-custody forms inside a plastic bag and tape to the inside of the cooler lid.
  - d. Place at least two custody seals across the lid and body of the cooler.

## **Groundwater Monitoring Equipment Checklist**

#### Instruments

Water level indicator pH/conductivity/temperature meter conductivity standard pH buffers **Operation manual** Landfill and well keys Bailers, if needed Rope, if needed 5-gallon bucket to measure purge volume, if required Cup to collect water for pH/conductivity/temperature measurements Sample bottles (provided by laboratory) Plastic tape for sample bottle labels Cooler and ice Gloves Paper towels Waterproof pens and pencils Field book Extra 9 volt battery Self-sealing freezer bags Knife **Plastic sheeting Deionized Water** Tygon tubing Peristaltic pump 0.45 micron filters Sampling and Analysis Plan (SAP) Pump controller Generator Forms

Sampling Field Data Sheets Chain-of-Custody

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# **Appendix B**

Bennett Road Landfill Quality Assurance Project Plan

# 2024 Bennett Road Landfill Quality Assurance Project Plan

Prepared for Elmore County



July 2024

# Parametrix

# 2024 Bennett Road Landfill Quality Assurance Project Plan

Prepared for

Elmore County 150 S 4th E Street Mountain Home, ID 83647

Prepared by

**Parametrix** 7761 W Riverside Drive, Suite 201 Boise, ID 83714-5044 T. 208.898.0012 F. 1.855.542.6353 www.parametrix.com

July 2024 | 553-7443-006

## Citation

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# Certification

The technical material and data contained in this document were prepared under the supervision and direction of the undersigned, whose seal, as a professional engineer licensed to practice as such, is affixed below.

Prepared by Shira DeGrood, PG

Checked by Michael Brady, LG, LHG

Approved by Lisa Gilbert, LG, LHG

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# **Acronyms and Abbreviations**

BRL	Bennett Road Landfill
CFR	Code of Federal Regulations
CLP	Contract Laboratory Program
DI	deionized
DQIs	data quality indicators
DQOs	data quality objectives
EPA	U.S. Environmental Protection Agency
HAZWOPER	Hazardous Waste Operation and Emergency Response
IDAPA	Idaho Administrative Code
LCS	laboratory control sample
MS	matrix spike
MSD	matrix spike duplicate
QA	quality assurance
QAPP	quality assurance project plan
RPD	relative percent difference
SOPs	standard operating procedures
Subtitle D	Federal Regulation 40, CFR Part 258, Solid Waste Disposal Facility Criteria

## **1.** Introduction

The purpose of this quality assurance project plan (QAPP) is to establish a system of quality and performance checks pertaining to collection of groundwater and surface water samples, laboratory analysis of samples, and reporting of results for the Bennett Road Landfill (BRL). This QAPP describes procedures to be used for sample collection and analysis and defines the data quality objectives (DQOs) and criteria for the project. Parametrix prepared this QAPP in accordance with the U.S. Environmental Protection Agency (EPA) requirements contained in the following:

- QA/R-5, EPA Requirements for Quality Assurance Project Plans (EPA 2001a).
- QA/G-5, EPA Guidance for Quality Assurance Project Plans (EPA 2002).

## 2. Project Management

#### **2.1 Project Organization**

Specific project quality assurance (QA) responsibilities for the BRL groundwater monitoring project are described in Table 2-1.

Personnel	Responsibilities
Project Manager Parametrix (206) 394-3700-6200	Coordinate field program and project-agency interaction with Elmore County. Oversee technical team performance to ensure successful accomplishment of the technical and quality assurance (QA) project objectives; review QA needs and approve QA corrective action where necessary.
Landfill Manager Elmore County Landfill Deb Ireland 208-943-1476 - mobile 208-943-1474 - office	Ensure that all field sampling and handling procedures are followed and documented; ensure that field QA objectives are met; coordinate and participate in the field sampling activities; report to the Project QA Officer any discrepancies or deviations from the QAPP.
Project QA Officer Parametrix (206) 394-3700	Direct implementation of QAPP, provide technical QA assistance, prepare QA Reports for the Project Manager, evaluate laboratory data, perform QA/quality control (QC), and prepare data validation reports.
Laboratory QA Officer Selected laboratory	Ensure that all laboratory QA objectives are met and data package QA/QC deliverables from the laboratory are correctly documented and reported.
Central District Health Department Brent Copes, REHS/RS Environmental Health Specialist Senior Community & Environmental Health 208-580-6004 – office 208-860-5469 - mobile <u>BCopes@cdh.idaho.gov</u> - email	Agency review and approval of groundwater monitoring plan and quarterly and annual reports.

Table 2-1. Quality Assurance Responsibilities, BRL Groundwater Monitoring Project

## 2.2 Project Description

This QAPP addresses the groundwater monitoring program for the BRL in Elmore County, Idaho. The facility is located approximately 6 miles southeast of Mountain Home along I-84. The address of the landfill is 6100 SE County Landfill Road, Mountain Home, Idaho, 83647.

### 2.3 Background

The groundwater monitoring approach is designed to meet the applicable federal and state regulations. The monitoring program will meet federal and state regulations for municipal waste landfills (Federal Regulation 40, Code of Federal Regulations [CFR] Part 258, Solid Waste Disposal Facility Criteria [Subtitle D], Idaho Administrative Code [IDAPA] 58.01.06, and the Idaho Solid Waste Facilities Act [§39-7410]).

## 2.4 Quality Objectives and Criteria

### 2.4.1 Data Quality Objectives

DQOs were developed according to EPA's DQOs Process (EPA 2000) to provide data of known and appropriate quality. The DQO process is a seven-step planning approach to develop sampling designs for data collection activities that support decision-making. It provides a systematic procedure for defining the criteria that a data collection design should satisfy. The DQOs for the project are shown in Table 2-2.

Data Quality Objective (DQO)	Description
State the Problem	Meet state and federal requirements for groundwater monitoring at municipals landfills to determine whether potential contaminants from the landfill are affecting groundwater quality.
Identify the Decisions	Are the contaminant levels above applicable groundwater quality criteria?
	Are contaminant levels increasing in detection monitoring wells?
Identify the Inputs to the Decisions	Analytical results (What are the detected concentrations? Are they above background levels? Were quality assurance/quality control [QA/QC] criteria met?).
Define the Study Boundaries	The landfill property boundaries and (possibly) downgradient areas.
Develop a Decision Rule	Results will be compared to Federal maximum contaminant levels (MCLs, from EPA Drinking Water Regulations), Idaho Regulations for Public Drinking Water Supplies, and Idaho Groundwater Quality Standards.
	Results will be compared to intrawell upper prediction limits (UPLs).
Specify Tolerable Limits on Decision Errors	The tolerable limits of uncertainty regarding the extent of contamination at the subject properties will be based on professional judgment.
	Tolerable limits on analytical results are determined by the QA/QC criteria defined in this QAPP.
Optimize the Design	Develop defensible groundwater monitoring locations, analytical procedures, and data analysis procedures to provide an early warning of groundwater contamination.
	Obtain appropriate qualitative limits for monitoring parameters so results can be compared to groundwater quality criteria.

#### Table 2-2. Sampling DQOs

## 2.4.2 Data Quality Indicators

Data quality and usability are evaluated in terms of performance criteria. Performance and acceptance criteria are expressed in terms of data quality indicators (DQIs). The principal indicators of data quality are precision, accuracy, bias, sensitivity, completeness, comparability, and representativeness. Table 2-3 provides a description of project DQIs.

Data Quality Indicator (DQI)	Description
Precision	A measure of agreement among repeated measurements of the same property under identical conditions. Usually assessed as a relative percent difference (RPD) between duplicate measurements. RPD guidelines for laboratory duplicate analyses are contained in the standard operating procedures (SOPs) for each analytical method and will be obtained from the laboratory for validation purposes.
Accuracy	A measure of the overall agreement of a measurement to a known value. Analytical accuracy is assessed as percent recovery from matrix spike or reference material measurements. Percent recovery guidelines are contained in laboratory SOPs for each analytical method.
Bias	The systematic or persistent distortion of a measurement process that causes error in one direction. Usually assessed with reference material or matrix spike measurements. Bias as reported by the laboratory will be used to assess data validity.
Sensitivity	The capability of a method or instrument to meet prescribed reporting limits. Assessed by comparison with risk-based reporting limits, method reporting limits, instrument reporting limits, or laboratory quantitation limits, as appropriate. In general, reporting limits for the analytical methods used will be at or below applicable criteria.
Completeness	A measurement of the amount of valid data needed to be obtained for a task. Assessed by comparing the amount of valid results to the total results set. Project requirements for completeness are 90%.
Comparability	A qualitative term that expresses the measure of confidence that one data set can be compared to another. Assessed by comparing sample collection and handling methods, sample preparation and analytical procedures, holding times, reporting units, and other QA protocols. To ensure comparability of data collected for the BRL to previous data, standard collection and measurement techniques will be used.
Representativeness	A qualitative term that expresses the degree to which data accurately and precisely represent a characteristic of a population, parameter variation at a sample point, or environmental condition. To ensure representativeness, the sampling design will incorporate sufficient samples so that contamination is detected, if present. Additionally, all sampling procedures detailed in this QAPP will be followed.

#### Table 2-3. General Description of DQIs

## 2.5 Special Training and Certification

All personnel conducting sampling activities on the project facility must be 40-hour Hazardous Waste Operation and Emergency Response (HAZWOPER) trained per 29CFR 1910.120 and be current with their annual 8-hour refresher course.

All personnel working at the project facility will be briefed on potential site hazards, health and safety procedures, and sampling procedures. Following completion of this training, all personnel will be required to sign an acknowledgment form verifying that they have completed the task-specific training.

## 2.6 Sampling Documentation and Records

Sampling and sample handling records to be used for groundwater and surface water sampling are listed in Table 2-4.

Record	Use	Responsibility/Requirements
Field Notebook	Record significant events and observations.	Maintained by field sampler/geologist; must be bound; all entries must be factual, detailed, objective; and entries must be signed and dated.
Sampling Field Data Sheet	Provide a record of each sample collected.	Completed, dated, and signed by sampler; maintained in project file.
Sample Label	Accompanies sample; contains specific sample identification information.	Completed and attached to sample container by sampler.
Chain-of-Custody Form	Provides a record of each sample number, date of collection/transport, sample matrix, analytical parameters for which samples are to be analyzed. Documents chain-of-custody for sample handing.	Completed by sampler at time of sampling/ transport; copies distributed to laboratory project file. Documented by sample number. Original accompanies sample. A copy is retained by the Project QA Officer.
Chain-of-Custody Seal	Seals sample shipment container (e.g., cooler) to prevent tampering or sample transference. Individual samples do not require custody seals, unless they are to be archived, before going to the lab for possible analysis at a later date.	Completed, signed, and applied by sampler at time samples are transported.
Sampling Container Request	Documents number of samples, analytical parameters, sampling dates.	Prepared by field coordinator and distributed to analytical laboratory prior to each sampling event.

Table 2-4. Sampling and Sample	Handling Records
--------------------------------	------------------

### 2.6.1 Field Logs and Forms

A bound field notebook will be maintained to provide daily records of significant events and observations that occur during field investigations. Pages of the field notebook are not to be removed, destroyed, or thrown away. Sampling Field Data Sheets will be used to document collection of each sample. All field logs and forms will be retained in the project files.

All entries are to be made in ink, signed, and dated. Corrections will be made by drawing a single line through the original entry (so that the original entry can still be read) and writing the corrected entry alongside. The correction will be initialed and dated. Most corrected errors will require a footnote explaining the correction. If an error made on a document is assigned to one person, that individual may make corrections simply by crossing out the error and entering the correct information. The erroneous information should not be obliterated. Any error discovered on a document should be corrected by the person who made the entry.

## 2.6.2 Photographs

All photographs taken of field activities will be documented with the following information noted in the field notebook:

- Date, time, and location of photograph taken.
- Description of photograph taken.
- Reasons photograph was taken.
- Viewing direction.

Digital photographs will be reviewed in the field to assess quality and the need to retake the photograph. For nondigital photographs, the photographer will review the photographs or slides when they return from developing, and compare them to the log, to assure that the log and the photographs match.

# **3.** Data Generation and Acquisition

## **3.1** Sampling Design

Groundwater samples will be collected from upgradient and downgradient wells. The monitoring wells are installed into the uppermost monitorable aquifer below the facility and are appropriately placed to detect potential impacts from leachate that may enter the underlying groundwater system.

## 3.2 Sampling Methods

Samples will be collected from groundwater wells at the facility. Sample locations and procedures for sample collection are specified in standard operating procedures in the groundwater sampling and analysis plan.

The following Table 3-1 provides a summary of sample analyses and specifications for containers, preservation, and holding times.

		-	-	-
Sample Container	<b>Container Size</b>	Preservation and Handling	Analyses	Holding Times 12
Glass vials; Teflon-lined silicon septum caps	(3) 40 ml	Fill leaving NO AIR SPACE, keep in dark on ice (4°C)	Volatile organics	7 days; 14 days if preserved
Glass or plastic bottle	500-1000 ml	HNO3 to pH < 2	Metals, unfiltered	6 months
Plastic bottle	500 ml	Keep on ice (4°C)	Chloride/Sulfate/ Nitrate/ Bicarbonate Alkalinity	28 days 7 days 48 hours 14 days
Plastic or glass beaker		In field	pH, temperature, specific conductivity	28 days

# Table 3-1. Sample Containers, Preparation, Preservatives, and Holding Times for Groundwater Samples

<sup>1</sup> APHA-AWWA-WPCF. 1989. Standard Methods for the Examination of Waste and Wastewater, 17th edition.

<sup>2</sup> U.S. Environmental Protection Agency. 1983. Methods for Chemical Analysis of Water and Wastes.

<sup>3</sup> U.S. Environmental Protection Agency. 1996. Test Methods for Evaluating Solid Waste (SW-846), 3rd Edition.

## 3.3 Sample Handling and Custody

This section describes standard operating procedures for sample custody and the chain-of-custody procedures to be used for this project. These procedures ensure that the quality and integrity of the samples are maintained during their collection, transportation, storage, and analysis.

Sample documents will be carefully prepared so that sample identification and chain-of-custody can be maintained and sample disposition controlled. Sample identification documents will include:

- Field notebooks.
- Sampling Field Data Sheets.
- Sample labels.
- Chain-of-custody records.

## 3.3.1 Chain-of-Custody

The chain-of-custody procedures used for this program provide an accurate written or computerized record that can be used to trace the possession of each sample from the time each is collected until completion of all required analyses. A sample is in custody if it is in any of the following places:

- In someone's physical possession.
- In someone's view.
- In a secured container.
- In a designated secure area.

The following information will be provided on the chain-of-custody form:

- Sample identification numbers.
- Matrix type for each sample.
- Analytical methods to be performed for each sample.
- Number of containers for each sample.
- Sampling date and time for each sample.
- Names of all sampling personnel.
- Signatures and dates indicating the transfer of sample custody.

#### 3.3.1.1 Field Custody Procedures

The following field custody procedures will be followed:

- As few people as possible will handle the samples.
- Coolers or boxes containing cleaned sample bottles will be sealed with a custody tape seal during transport to the field or while in storage before use.
- The sample collector will be responsible for the care and custody of the samples collected until the samples are transferred or dispatched properly.
- The sample collector will record sample data on the sample collection form.
- The field coordinator will determine whether proper custody procedures were followed during the field work and will decide if additional samples are required.

#### 3.3.1.2 Laboratory Custody Procedures

A designated sample custodian will accept custody of the shipped samples and verify that the information on the sample labels matches the chain-of-custody records. Pertinent information on shipment, pickup, courier, and condition of the samples will be entered in the Remarks section of the chain-of-custody form. The custodian will enter the sample identification number data into a logbook, which is arranged by project code and station number.

The laboratory custodian will use the sample identification number or assign a unique laboratory number to each sample and then transfer the samples to the proper analyst or store them in the appropriate secure area. Sample control and custody at the laboratory through sample disposal will be conducted in accordance with standard laboratory procedures that maintain the sample integrity and security.

### 3.3.2 Transfer of Custody and Shipment

When samples are transferred, the person relinquishing the samples will sign the chain-of-custody record and record the date and time of transfer. The sample collector will sign the form in the first signature space.

Program documentation of sample custody will be verified by the Project QA Officer during regular review of the data validation package.

The following transfer of custody and shipment procedures will be followed:

- The coolers in which samples are packed must be accompanied by a chain-of-custody record. When transferring samples, the individuals relinquishing and receiving them will sign, date, and note the time on the chain-of-custody record to document sample custody transfer.
- Shipping containers will be sealed with custody seals for shipment to the laboratory. The method of shipment, name of courier, and other pertinent information will be entered in the Remarks section of the chain-of-custody record.
- All shipments will be accompanied by the chain-of-custody record identifying their contents. The original record will accompany the shipment. The other copies will be distributed as appropriate to the Project QA Officer and Project Manager.
- If sent by mail, the package will be registered with return receipt requested. If sent by common carrier, a bill of lading will be used. Freight bills, postal services receipts, and bill of lading will be retained as part of the permanent documentation.

### 3.3.3 Sample Identification

Each sample will be labeled, chemically preserved (if required), and sealed immediately after collection. The labels will be filled out using waterproof ink and then firmly affixed to the sample containers and protected with clear, water-resistant tape.

The following information will be given on each sample label:

- Project name and number.
- Name of sampler.
- Date, time, and location of collection.
- Sample designation.
- Analysis required.
- Preservative, if any.

### 3.3.4 Sample Packaging and Shipping

The samples will be transported and handled in a manner that not only protects the integrity of the samples, but also prevents any detrimental effects due to the possible hazardous nature of the samples. Samples will routinely be shipped to the analytical laboratory within 24 hours of sample collection.

## 3.4 Analytical Methods

### 3.4.1 Analytical Methods and Quantitation Limits

Analytical methods and quantitation limits for the planned analyses are presented in Table 3-2. Quantitation limits will be below applicable groundwater quality criteria, if possible, using conventional analytical methods. Quality control checks and decision criteria for determining if an analysis is within quality control requirements will follow the quality control procedures and guidelines listed in SW-846 (EPA 1996).

Where appropriate, these procedures may be modified based on anticipated data uses and with recognition of validation requirements, to incorporate techniques familiar to the project laboratory. The laboratory will notify the Project QA Officer of any proposed procedural changes and document these changes in the cover letter with the data reports.

Matrix interferences may make achievement of the desired detection limits and associated quality control criteria impossible. In such instances, the laboratory must report to the Project QA Officer the reason for noncompliance with quality control criteria or elevated detection limits.

Parameters:	Units	Analytical Method	Quantitation Limit (QL)	MCL	IDAPA	
Conventionals						
Bicarbonate Alkalinity	mg/L	SM 2320B	20			
Calcium	mg/L	6010	0.5			
Magnesium	mg/L	6010	0.5			
Sodium	mg/L	6010	10			
Potassium	mg/L	6010	0.5			
Nitrate	mg/L	EPA 9056A	0.01	10	10	
Chloride	mg/L	EPA 9056A	1	250 **	250 **	
Sulfate	mg/L	EPA 9056A	2	250 **	250 **	
Metals						
Antimony	mg/L	200.8	0.0002	0.006	0.006	
Arsenic	mg/L	200.8	0.0002	0.01	0.05	
Barium	mg/L	6010	0.006	2	2	
Beryllium	mg/L	6010	0.001	0.004	0.004	
Cadmium	mg/L	6010	0.009	0.005	0.005	
Chromium	mg/L	6010	0.005	0.1	0.1	
Cobalt	mg/L	6010	0.003			
Copper	mg/L	6010	0.003	1.0 **	1.3	
Lead	mg/L	200.8	0.0001	0.015***	0.015	
Nickel	mg/L	6010	0.010			
Selenium	mg/L	200.8	0.0005	0.05	0.05	
Silver	mg/L	6010	0.003	0.1 **	0.1 **	
Thallium	mg/L	200.8	0.0002	0.002	0.002	
Vanadium	mg/L	6010	0.003			

#### Table 3-2. Proposed Methods and Anticipated Quantitation Limits for Analysis of Groundwater

Parameters:	Units	Analytical Method	Quantitation Limit (QL)	MCL	IDAPA
Zinc	mg/L 6010 0.020		0.020	5 **	5 **
Volatile Organics					
1,1,1,2-Tetrachloroethane	µg/L	SW 8260D	0.2		
1,1,1-Trichloroethane	µg/L	SW 8260D	0.2	200	200
1,1,2,2-Tetrachloroethane	µg/L	SW 8260D	0.2		
1,1,2-Trichloroethane	µg/L	SW 8260D	0.2	5	5
1,1-Dichloroethane	µg/L	SW 8260D	0.2		
1,1-Dichloroethene	µg/L	SW 8260D	0.2	7	7
1,2,3-Trichloropropane	µg/L	SW 8260D	0.5		
1,2-Dibromo-3-chloropropane	µg/L	SW 8260D	0.5	0.2	0.2
1,2-Dibromoethane	µg/L	SW8260D	0.2	0.05	0.05
1,2-Dichlorobenzene	µg/L	SW 8260D	0.2	600	600
1,2-Dichloroethane (total)	µg/L	SW 8260D	0.2	5	5
1,2-Dichloropropane	µg/L	SW 8260D	0.2	5	5
1,4-Dichlorobenzene	µg/L	SW 8260D	0.2	75	75
2-Butanone	µg/L	SW 8260D	5.0		
2-Hexanone	µg/L	SW 8260D	5.0		
4-Methyl-2-pentanone	µg/L	SW 8260D	5.0		
Acetone	µg/L	SW 8260D	5		
Acrylonitrile	µg/L	SW 8260D	1.0		
Benzene	µg/L	SW 8260D	0.2	5	5
Bromochloromethane	µg/L	SW 8260D	0.2		
Bromodichloromethane	µg/L	SW 8260D	0.2	80 THM	100
Bromoform	µg/L	SW 8260D	0.2	80 THM	100
Bromomethane	µg/L	SW 8260D	1.0		
Carbon disulfide	µg/L	SW 8260D	0.2		
Carbon tetrachloride	µg/L	SW 8260D	0.2	5	5
Chlorobenzene	µg/L	SW 8260D	0.2	100	100
Chloroethane	µg/L	SW 8260D	0.2		
Chloroform	µg/L	SW 8260D	0.2	80 THM	2
Chloromethane	µg/L	SW 8260D	0.5		
cis-1,2-Dichloroethene	µg/L	SW 8260D	0.2	70	70
cis-1,3-Dichloropropene	µg/L	SW 8260D	0.2		
Dibromochloromethane	µg/L	SW 8260D	0.2	80 THM	100
Dibromomethane	µg/L	SW 8260D	0.2		
Ethylbenzene	µg/L	SW 8260D	0.2	700	700
m,p-xylene	µg/L	SW 8260D	0.4	10,000 XYL	10,000
Methyl lodide	µg/L	SW 8260D	1.0		
Methylene chloride	µg/L	SW 8260D	1.0	5	5
o-xylene	µg/L	SW 8260D	0.2	10,000 XYL	10,000
Styrene	µg/L	SW 8260D	0.2	100	100
Tetrachloroethene	µg/L	SW 8260D	0.2	5	5
Toluene	µg/L	SW 8260D	0.2	1,000	1,000

Parameters:	Units	Analytical Method	Quantitation Limit (QL)	MCL	IDAPA
Trans-1,2-Dichloroethene	µg/L	SW 8260D	0.2	100	100
Trans-1,3-Dichloropropene	µg/L	SW 8260D	0.2		
Trans-1,4-Dichloro-2-butene	µg/L	SW 8260D	1.0		
Trichloroethene	µg/L	SW 8260D	0.2	5	5
Trichlorofluoromethane	µg/L	SW 8260D	0.2		
Vinyl Acetate	µg/L	SW 8260D	0.2		
Vinyl Chloride	µg/L	SW 8260D	0.2	2	2

MCL = Federal Maximum Contaminant Level, EPA Drinking Water Regulations

IDAPA = Idaho Regulations for Public Drinking Water Systems (IDAPA 58.01.08) and Idaho Groundwater Quality Standards (IDAPA 58.01.11)

\*\* = National Secondary Drinking Water Standard

\*\*\* = Action Level

XYL = Primary MCL for the sum of all xylenes

THM = Primary MCL for the sum of all trihalomethanes

## 3.4.2 Data Reporting

All laboratory data packages will contain the following information:

- Cover letter.
- Chain-of-custody forms.
- Summary of sample results.
- Summary of quality control (QC) results.

The information provided in the cover letter will include:

- Laboratory name, address, and telephone number.
- Date(s) of sample receipt and number of samples received.
- Detailed description of any problems encountered with QC, analysis, shipment or handling procedures.
- Identification of possible reasons for any QC criteria outside acceptance limits.
- Signature of laboratory representative and date certifying data results.

The minimum information to be presented for each sample for each parameter or parameters group will include:

- Client sample number and laboratory sample number.
- Sample matrix.
- Date of extraction/preparation and date/time of analysis.
- Dilution factors.
- Sample weights/volumes used in sample preparation/analysis.
- Identification of analytical instrument.
- Analytical method.

- Detection/quantitation limits.
- Definitions of any data qualifiers used.

The minimum QC summary information to be presented for each sample and for each parameter or parameter group will include:

- Surrogate standard recovery results.
- Matrix QC results (matrix spike/matrix spike duplicate, duplicate).
- Method blank results.
- Laboratory check standard results.

## 3.5 Quality Control

Quality control checks will consist of measurements performed in the field and laboratory. The analytical methods referenced in Section 3.4 specify routine methods required to evaluate data precision and accuracy and determine whether the data are within the quality control limits. Guidelines for minimum samples for field QA/QC sampling and laboratory analysis are summarized in Table 3-3.

	Eaboratory Analysis of Water Samples							
Field					Laboratory			
Field Replicate	Field Rinsate Blank <sup>1</sup>	Trip Blank <sup>2</sup>	Matrix Duplicate <sup>3</sup>	Matrix Spikes	Matrix Spike Duplicate <sup>4</sup>	Method Blanks	LCS ⁵	
1 in 20 6	1 in 20	1 per cooler	1 in 20	1 in 20	1 in 20	1 in 20	1 in 20	

Table 3-3. Guidelines for Minimum QA/QC Samples for Field Sampling and
Laboratory Analysis of Water Samples

1 Field rinsate blanks are not required for dedicated or disposable equipment.

2 Trip Blank analyzed for volatile organic compounds only.

3 Matrix duplicate analyzed for metals.

4 Matrix spike duplicates analyzed for organic analyses.

5 LCS = Laboratory Control Sample.

6 All frequencies of 1 in 20 indicate 1 per batch, when the batch is less than 20.

## 3.5.1 Field QC Samples

The following quality control samples will be evaluated to verify accuracy and precision of laboratory results for this project. The frequency of quality control sample evaluation is also indicated by sample type but may be adjusted when the final sampling schedule is determined. The frequencies of quality control sample evaluation described here should be considered a minimum.

#### 3.5.1.1 Rinsate Blank

One rinsate blank will be analyzed for every 20 samples of a similar matrix (groundwater, surface water), or one per sampling event, whichever is greater. If the equipment used for sampling is dedicated equipment (not reused to obtain other samples), no rinsate blank is necessary.

Rinsate blanks will consist of deionized (DI) or distilled water (supplied by the analytical laboratory) poured over and/or through the sampling equipment after decontamination. Surfaces and materials exposed during actual sampling will be rinsed to evaluate the effectiveness of sampling equipment decontamination procedures and the potential for sample cross-contamination in the field.

#### 3.5.1.2 Trip Blank

There will be one trip blank in each cooler used to ship volatile organic samples to the laboratory. The trip blank will consist of a purged-free DI/distilled water blank supplied by the analytical laboratory. It will be transported to and from the field, then returned to the laboratory unopened and unaltered for analysis. The term "purged-free" water refers to DI/distilled water that has been boiled and capped in the laboratory.

#### 3.5.1.3 Transfer Blank

Transfer blanks will be performed and analyzed if the source of trip blank and rinsate blank contamination cannot be discovered. The transfer blank will consist of DI/distilled water (supplied by the analytical laboratory) transferred in the field into the appropriate sampling containers. The transfer blank will evaluate possible sample contamination from the field.

#### 3.5.1.4 Field Duplicate

A minimum of one field duplicate will be analyzed per 20 samples or one per sampling event (whichever is greater), to verify the precision of laboratory and/or sampling methodology.

### 3.5.2 Laboratory QC Samples

Specific procedures and frequencies for laboratory quality control are detailed by analytical method in the laboratory QA plan. A general description of the types of required laboratory QC samples is provided in the following sections.

#### 3.5.2.1 Method Blank

A minimum of one method blank will be analyzed per 20 samples or one per batch (whichever is greater), to assess possible laboratory contamination. Method blanks will contain all reagents and undergo all procedural steps used for analysis.

#### 3.5.2.2 Laboratory Control Sample

A minimum of one laboratory control sample (LCS) will be analyzed per 20 samples or one per sampling event (whichever is greater), to verify precision of laboratory equipment. The LCS will be a concentration within the calibration range at a different concentration than the standards used to establish the calibration curve. LCS analysis will follow EPA LCS guidelines established in SW-846 (EPA 1996).

#### 3.5.2.3 Matrix Spike

A minimum of one matrix spike (MS) will be analyzed per 20 samples or one per sampling event (whichever is greater), to monitor recoveries and to ensure that extraction and concentration levels are acceptable. The matrix spike will be analyzed on a separate water sample collected at a well or surface water station. The matrix spike will follow the matrix spike guidelines specified in the Contract Laboratory Program (CLP) statements of work (EPA 1993a, 1993b).

#### 3.5.2.4 Matrix Spike Duplicate

A minimum of one matrix spike duplicate (MSD) per 20 samples will be analyzed for volatile organics, or one per sampling event (whichever is greater), to provide information on the precision of chemical analysis. The matrix spike duplicate will be analyzed on a separate water sample collected

at the same sampling station from which the matrix spike is collected. MSDs (rather than matrix duplicates) are analyzed for organic analyses, because of the large number of undetected compounds. Comparing the MS and MSD provides better information on the quality of the data. The MSD will follow EPA MSD guidelines specified in SW-846 (EPA 1996).

#### 3.5.2.5 Matrix Duplicate

A minimum of one laboratory matrix duplicate will be analyzed per 20 samples, or one per sampling batch (whichever is greater), when samples are analyzed for metals or conventional parameters to provide information on the precision of chemical analysis. The matrix duplicate will follow EPA duplicate guidelines specified in SW-846 (EPA 1996).

## **3.6** Instrument Testing, Inspection, and Maintenance

### 3.6.1 Field Instruments

The field coordinator will arrange for field instrumentation preventive maintenance. Preventive maintenance on field instruments will be performed by qualified field technicians following the manufacturer's instructions and maintenance schedules. Maintenance will be documented in instrument log books with the date and initials of the individual performing the maintenance.

The field coordinator will routinely review and compare instrument calibration results against the preventive maintenance records to verify the effectiveness of the maintenance program. The field coordinator will track scheduling of maintenance required by the manufacturer.

### 3.6.2 Laboratory Instruments

The analytical laboratory manager is ultimately responsible for the care of the laboratory instruments. The manager may delegate the responsibility to the senior supervising chemists or technician qualified to perform routine maintenance, after demonstrating that personnel are trained in maintenance procedures for that laboratory section (wet chemistry, metals, and organics). Training of laboratory personnel on the routine care of laboratory equipment will be provided, at a minimum, during the initial installation of the equipment and, for new analysts, before initial use of the equipment.

Maintenance and other appropriate details will be documented in daily maintenance logbooks. The individual performing the maintenance procedures will date and sign each entry. At a minimum, the preventive maintenance schedules contained in the EPA methods and in the equipment manufacturer's instructions will be followed.

## **3.7** Instrument/Equipment Calibration and Frequency

### 3.7.1 Field Instruments

Field instruments will be calibrated according to manufacturer's instructions. All field instruments to be used will be calibrated on a daily basis. The following data will be recorded in the field notebook or on appropriate field forms:

- Date.
- Project number.
- Instrument make/model number.

- Calibration gas cylinder serial number (if applicable).
- Instrument response during calibration.

### 3.7.2 Laboratory Instruments

All instruments and equipment used during analysis will be operated, calibrated, and maintained according to manufacturer's guidelines and recommendations, and in accordance with procedures in the analytical method cited, as documented in the laboratory QA Plan. Properly trained personnel will operate, calibrate, and maintain laboratory instruments. Calibration blanks and check standards will be analyzed daily for each parameter to verify instrument performance and calibration before beginning sample analysis.

Where applicable, all calibration procedures will meet or exceed EPA CLP protocols (EPA 1993a, 1993b). Any variations from these procedures must be approved by the Project QA Officer before beginning sample analysis.

After the instruments are calibrated and standardized within acceptable limits, precision and accuracy will be evaluated by analyzing a QC check sample for each analysis performed that day. Acceptable performance of the QC check sample verifies the instrument performance on a daily basis. Analysis of a QC check standard is also required. QC check samples containing all analytes of interest will be either purchased commercially or prepared from pure standard materials independently from calibration standards. The QC check samples will be analyzed and evaluated according to the EPA method criteria.

Instrument performance check standards and calibration blank results will be recorded in a laboratory instrument log book, which will also contain evaluation parameters, benchmark criteria, and maintenance information. If the instrument log book does not provide maintenance information, a separate maintenance log book must be maintained for the instrument.

## **3.8** Inspection/Acceptance of Supplies and Consumables

Field supplies such as sample containers and trip/rinsate blank water shall be obtained from reputable suppliers and shall be certified analyte free. Records of certification shall be kept by the laboratory (for laboratory-supplied supplies) or by Parametrix in the project file. Sampling spoons and bowls shall be food-grade and shall be purchased new.

## 3.9 Nondirect Measurements

The need for nondirect measurements is not anticipated for the BRL. However, if the need does arise during task execution, the previously collected data will be evaluated to assess consistency with project DQOs and DQIs. Data from nondirect sources will be evaluated by the Project QA Officer prior to the data being used in analyses or in data reports.

## 3.10 Data Management

The objectives of data management are to ensure that large volumes of information and data are technically complete, accessible, and efficiently handled.

## 3.10.1 Field Data

The original field notebook, sampling data sheets, chain-of-custody forms, and field equipment calibration sheets will be stored in the project file. Photocopies of these documents should be prepared for working copies as needed.

### 3.10.2 Laboratory Data

The laboratory data reports will be archived in the project files. The electronic data will be incorporated into Excel spreadsheets and archived on electronic media and placed in the project file.

# 4. Assessment and Oversight

This section describes activities to be conducted to assess the effectiveness of project implementation and associated QA/QC activities. The purpose of the assessment is to ensure that the QAPP is properly implemented.

## 4.1 Assessments and Response Actions

A performance and system audit may be conducted at the discretion of the Project Manager. Audits will consist of direct observation of work being performed and inspection of field and laboratory equipment. The performance and system audits will also review the sample custody procedures in the field and laboratory.

If implemented, internal audits of both the field and laboratory activities will be conducted by the Project QA Officer. Audits will be unannounced to ensure a true representation of the technical and QA procedures employed.

Checklists for both field and laboratory audits will be based on National Enforcement Investigation Center (EPA 1984) Audit Checklists. The audits will be performed by persons having no direct responsibilities for the activities being performed.

The auditor or designee will prepare an audit report that includes findings, nonconformances, observations, recommended corrective action, and a schedule for completion of such action.

For each identified nonconformance, a corrective action report will be issued as part of the audit report to notify the individual responsible for implementing the recommended corrective action and its schedule for completion. If a field corrective action is required, the Project Manager will be notified. If a laboratory corrective action is required, the Project QA Officer will be notified. The audit will be distributed to the Project Manager.

Corrective actions may be needed for two categories of nonconformance:

- Deviations from the methods or QA requirements established in the QAPP.
- Equipment or analytical malfunctions.

During field operations and sampling procedures, the field sampler will be responsible for taking and reporting required corrective action. A description of any such action taken will be entered in the field notebook. If field conditions are such that conformance with the QAPP is not possible, the Project Manager will be consulted immediately. Any corrective action or field condition resulting in a major revision of the QAPP will be communicated to the Project Manager for review and concurrence.

During laboratory analysis, the Laboratory QA Officer will be responsible for taking required corrective actions in response to equipment malfunctions. If an analysis does not meet data quality goals outlined in the QAPP, corrective action will follow the guidelines in SW-846 (EPA 1996). If analytical conditions do not conform to this QAPP, the Project QA Officer will be notified as soon as possible so that additional corrective actions can be taken.

Corrective action reports will document response to any reported nonconformances. These reports may be generated from internal or external audits or from informal reviews of project activities. Corrective action reports will be reviewed for appropriateness of recommendations and actions by the Project QA Officer for QA matters, and the Project Manager for matters of technical approach.

## 4.2 Reports to Management

A QA data validation report will be prepared for all data packages. This QA report will summarize all relevant data quality information. The Project QA Officer will be responsible for data quality assessments and associated QA reports. Final task or investigative reports will contain a separate QA section summarizing data quality information.

# 5. Data Verification and Validation

Data verification is confirmation by examination and provision of objective evidence that specified requirements have been fulfilled. Validation is confirmation by examination and provision of objective evidence that the particular requirement for a specific intended use has been fulfilled. Techniques for data verification and validation will be in accordance with the Guidance on Environmental Data Validation and Verification (EPA 2001b) and National Functional Guidelines (EPA 2020a, 2020b).

## 5.1 Data Review, Verification, And Validation

Analytical data will be reviewed by the Laboratory QA Officer to ensure that the QA/QC objectives for precision, accuracy, representativeness, completeness, and comparability have been met. These reviews will identify the occurrence of deficiencies in time to take corrective action. If the required QC objectives are not met after the corrective action is performed, the Project Manager will be notified by the laboratory before data submittal. The Project Manager and Project QA Officer will determine if additional corrective action should be taken, such as reanalysis, if applicable. All data packages provided by the laboratory must provide a summary of QC results adequate to enable reviewers to determine the quality of the data.

The Project QA Officer is responsible for conducting checks for internal consistency, transmittal errors, and for adherence to the QC elements. The Project QA Officer will review the data package submitted by the laboratory to ensure that documentation has been provided, appropriate QC checks have been performed, and that appropriate corrective actions have been taken. The Project QA Officer will then determine the potential effects of any deviations or corrective actions on the suitability of the data.

## 5.2 Verification and Validation Methods

The Project QA Officer will review the following:

- Chain-of-custody documentation.
- Holding times.
- Equipment/trip blank results.
- Field duplicate results.
- Method blank results.

A limited review (minimum 10%) of the following laboratory QC data results will be conducted:

- Laboratory MS/MSD and/or matrix duplicate results.
- Laboratory surrogate recoveries.
- Laboratory check samples.

If, based on this limited review, the QC data results indicate potential data quality problems, further evaluations will be conducted.

## 5.2.1 Precision

Precision measures the mutual agreement among individual measurements of the same property, usually under prescribed similar conditions. QA/QC sample types that measure precision include field duplicates, MSD, and matrix duplicates. The estimate of precision of duplicate measurements is expressed as a RPD, which is calculated:

$$RPD = \frac{D_1 - D_2}{(D_1 + D_2) \div 2} x \ 100$$

Where D1 = First sample value D2 = Second sample value.

The RPDs will be routinely calculated and compared with DQOs.

### 5.2.2 Accuracy

Accuracy is assessed using the results of standard reference material, linear check samples, and MS analyses. It is normally expressed as a percent recovery, which is calculated:

Percent Recovery = (Total Analyte Found - Analyte Originally Present) x 100 Analyte Added

The percent recovery will be routinely calculated and checked against DQOs.

### 5.2.3 Bias

Bias is the systematic or persistent distortion of a measurement process that causes errors in one direction. Bias will be assessed with field duplicate and laboratory matrix spike samples, similar to that described for accuracy. Bias measurements are usually carried out with a minimum frequency of 1 in 20, or one per batch of samples analyzed, under the same sampling episode.

### 5.2.4 Sensitivity

Sensitivity expresses the capability of a method or instrument for meeting prescribed measurement reporting limits. Sensitivity will be assessed by comparing data reporting limits with applicable cleanup criteria and analytical or instrument method reporting limits.

### 5.2.5 Completeness

The amount of valid data produced will be compared with the total analyses performed to assess the percent of completeness. Completeness will be routinely calculated and compared with the DQOs.

### 5.2.6 **Comparability**

Comparability is a qualitative parameter expressing the confidence with which one data set can be compared with another. Sample data will be comparable with other measurement data for similar samples and sample conditions. Comparability of the data will be maintained by using consistent methods and units.

## 5.2.7 Representativeness

Sample locations and sampling procedures will have been chosen to maximize representativeness. A qualitative assessment (based on professional experience and judgment) will be made of sample data representativeness based on review of sampling records and QA audit of field activities.

## 5.3 **Reconciliation and User Requirements**

The Project QA Officer will prepare a technical memorandum for each data package describing the results of the data review and describing any qualifiers that were added to the data. The technical memorandum will include recommendations on whether additional actions such as resampling are necessary.

Corrective actions may be needed for two categories of nonconformance:

- Deviations from the methods or QA requirements established in the QAPP or groundwater monitoring plan.
- Equipment or analytical malfunctions.

During field operations and sampling procedures, the project field coordinator will be responsible for taking and reporting required corrective action. A description of any such action taken will be entered in the field notebook. If field conditions are such that conformance with the QAPP or the groundwater monitoring plan is not possible, the Project QA Officer will be consulted immediately. Any corrective action or field condition resulting in a major revision of the QAPP or groundwater monitoring plan will be communicated to the Project Manager for review and concurrence. This communication will be made before changes in the field activities whenever possible.

During laboratory analysis, the Laboratory QA Officer will be responsible for taking required corrective actions in response to equipment malfunctions. If an analysis does not meet data quality goals outlined in the QAPP, corrective action will follow the guidelines in SW-846 (EPA 1996). If analytical conditions do not conform with this QAPP, the Project QA Officer will be notified as soon as possible so that any additional corrective actions can be taken.

Corrective action reports will document response to any reported nonconformances. These reports may be generated from internal or external audits or from informal reviews of project activities.

Corrective action reports will be reviewed for appropriateness of recommendations and actions by the Project QA Officer for QA matters, and the Project Manager for matters of technical approach.

# 6. References

- APHA (American Public Health Association), AWWA (American Water Works Association), and WPCF (Water Pollution Control Federation). 1989. Standard methods for the examination of waste and wastewater, 17th edition.
- EPA (U.S. Environmental Protection Agency). 1983. Methods for Chemical Analyses for Water and Wastes. U.S. Environmental Protection Agency, Washington, D.C.
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- EPA. 1993a. Statement of Work for Inorganic Analysis, Multi-Media, Multi-Concentration (ILM 03.0). U.S. EPA Contract Laboratory Program, U.S. Environmental Protection Agency, Washington, D.C.
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- EPA. 2001a. EPA Requirements for Quality Assurance Project Plans. EPA QA/R-5, EPA/240/B-01/003, March.
- EPA. 2001b. Guidance on Environmental Data Validation and Verification. EPA QA/G-8.
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- EPA. 2020a. National Functional Guidelines for Inorganic Superfund Data Review. EPA 540R- 2017-001. November.
- EPA. 2020b. National Functional Guidelines for Organic Superfund Data Review. EPA 542-R-20-006. November.